STORMWATER POLLUTION PREVENTION PLAN & DRAINAGE ANALYSIS

SPCA of Westchester 590 North State Road Town of Ossining - New York

October 9, 2018



Hudson Engineering & Consulting, P.C.

45 Knollwood Road - Suite 201 Elmsford, NY 10523

Table of Contents

- 1) NYSDEC Notice of Intent
- 2) Contractor Certification Statement
- 3) Narrative:
 - A. Introduction
 - B. Methodology
 - C. List of Permits
 - D. Pre-Design Investigative Analysis
 - E. Pre-Developed Condition
 - F. Post-Developed Condition
 - G. Summary of Flows at Design Point
 - H. Water Quality Volume
 - I. NYSDEC Table 3.1 Design Regulations
 - J. Construction Phase
 - K. Construction Sequencing
 - L. Erosion and Sediment Control Components
 - M. Construction Practices to Minimize Stormwater Contamination
 - N. Stormwater Management Facilities Maintenance Program
 - O. Conclusion
- 4) Extreme Precipitation Table
- 5) Soils Report
- 6) Watershed Maps
- 7) Pre-Development Calculations 1- 10-, 25- and 100-Year Storm Events
- 8) Post-Development Calculations 1-, 10-, 25- and 100-Year Storm Events
- 9) Water Quality Calculations
- 10) State of Maryland Department of the Environment (MDE) List of Approved Stormwater Practices (August 2017).
- 11)Stormwater Management Construction Checklists:

- A. Construction Site Log Book
- **B. Monthly Summary of Site Inspection Activities**
- **C.** Inspection and Maintenance Checklist
 - Catch Basins, Manholes, and Inlets
 - Conveyance Systems (Pipes & Ditches)
 - Vaults, Tanks, and Attenuation Piping

1.) NYSDEC Notice of Intent

NOTICE OF INTENT



New York State Department of Environmental Conservation Division of Water

625 Broadway, 4th Floor

NYR					
	(for	DEC	use	onl	y)

Albany, New York 12233-3505

Stormwater Discharges Associated with Construction Activity Under State Pollutant Discharge Elimination System (SPDES) General Permit # GP-0-15-002 All sections must be completed unless otherwise noted. Failure to complete all items may result in this form being returned to you, thereby delaying your coverage under this General Permit. Applicants must read and understand the conditions of the permit and prepare a Stormwater Pollution Prevention Plan prior to submitting this NOI. Applicants are responsible for identifying and obtaining other DEC permits that may be required.

-IMPORTANT-RETURN THIS FORM TO THE ADDRESS ABOVE

OWNER/OPERATOR MUST SIGN FORM

Owner/Operator Information														
Owner/Operator (Company	Name/Private Owner Name/	/Municipality Name)												
Owner/Operator Contact F	Person Last Name (NOT CON	NSULTANT)												
Owner/Operator Contact F	Person First Name													
Owner/Operator Mailing A	Address													
City														
State Zip	-													
Phone (Owner/Operator)	Fax (Owner/Op	erator)												
Email (Owner/Operator)														
FED TAX ID														
	(not required for individual	duals)												

	Project Site Inform	mation	
Project/Site Name			
Street Address (NOT P.O. BOX)			
Side of Street North South East Wes	t		
City/Town/Village (THAT ISSUES E	BUILDING PERMIT)		
State Zip	County	DEC Re	gion
Name of Nearest Cross Street			
Distance to Nearest Cross Street	(Feet)	Project In Relation to Cro O North O South O East	_
Tax Map Numbers Section-Block-Parcel		Tax Map Numbers	
1. Provide the Geographic Coord must go to the NYSDEC Stormwa			this you
www.dec.ny.gov/ims	maps/stormwater/view	wer.htm	
Zoom into your Project Location your site. Once you have locate choose "i"(identify). Then cli the X, Y coordinates in UTM wil below. For problems with the in	d your project site, ck on the center of l pop up. Transcribe	, go to the tool boxes on the your site and a new window c these coordinates into the	top and ontaining
X Coordinates (Easting)	Y Coordinates (Northing)	
2. What is the nature of this o	construction project	?	
O New Construction			
	vith increase in imposith no increase in s		

3. Select the predominant land use for both pre and post development conditions. SELECT ONLY ONE CHOICE FOR EACH

Pre-Development Existing Land Use	Post-Development Future Land Use
○ FOREST	O SINGLE FAMILY HOME Number of Lots
O PASTURE/OPEN LAND	O SINGLE FAMILY SUBDIVISION
○ CULTIVATED LAND	O TOWN HOME RESIDENTIAL
○ SINGLE FAMILY HOME	O MULTIFAMILY RESIDENTIAL
○ SINGLE FAMILY SUBDIVISION	○ INSTITUTIONAL/SCHOOL
O TOWN HOME RESIDENTIAL	○ INDUSTRIAL
○ MULTIFAMILY RESIDENTIAL	○ COMMERCIAL
○ INSTITUTIONAL/SCHOOL	O MUNICIPAL
○ INDUSTRIAL	○ ROAD/HIGHWAY
○ COMMERCIAL	○ RECREATIONAL/SPORTS FIELD
○ ROAD/HIGHWAY	O BIKE PATH/TRAIL
O RECREATIONAL/SPORTS FIELD	○ LINEAR UTILITY (water, sewer, gas, etc.)
○ BIKE PATH/TRAIL	O PARKING LOT
○ LINEAR UTILITY	O CLEARING/GRADING ONLY
O PARKING LOT	O DEMOLITION, NO REDEVELOPMENT
○ OTHER	○ WELL DRILLING ACTIVITY *(Oil, Gas, etc.)
	OTHER
*Note: for gas well drilling, non-high volume	hydraulic fractured wells only
4. In accordance with the larger common plan of enter the total project site area; the total existing impervious area to be disturbed (factivities); and the future impervious area disturbed area. (Round to the nearest tenth	l area to be disturbed; for redevelopment constructed within the of an acre.)
	Future Impervious ing Impervious Area Within To Be Disturbed Disturbed Area
5. Do you plan to disturb more than 5 acres of	soil at any one time? O Yes O No
6. Indicate the percentage of each Hydrologic A B B	Soil Group(HSG) at the site. C D %
7. Is this a phased project?	\bigcirc Yes \bigcirc No
8. Enter the planned start and end dates of the disturbance activities.	te

area?

	_																																											_	
9.		Ide dis					е	ne	aı	es	st	sı	ır	fa	ce	Wa	at	er]	bo	dy	(i	es)	t	-0	wh	iic	ch	CO	ns	tr	uc	:t:	Lor	ı	sit	ce	rı	ın	of	f	wi	11			
Nan	ne																																												
							T														I																								
							T					Τ	T	T							T		T								T					T									٦
											-		_																		_														
	a. O		_	_							_											ion nsw			b)																				
	0	We	tl:	anc	l /	/ 5	St	ate	≘	Ju	ri	sd	ic	cti	.01	n C	ff	= 5	Sit	:e																									
	0	We	tl	and	l /	/ F	'e	dei	ra	1 .	Ju	ri	sc	lic	:t:	ion	ı C	n	Si	lte	:	(An	sw	er	9	b)																			
	0	We	tl	and	l /	/ F	'e	dei	ra	1 .	Ju	ri	sc	lic	:t:	ion	ı C)ff	: 5	Sit	.e																								
	0	St	re	am	/	Cr	re	ek	0	n	Si	te	!																																
	0	St	re	am	/	Cr	re	ek	0	ff	S	it	e																																
	0	Ri	ve:	r C)n	Si	.t	e																																					
	0	Ri	ve:	r C)ff	= 5	Si	te															-	9b.		Η	low	7 W	as	t	he	. V	vet	:18	anc	i i	id€	en	ti:	fi	ed'	?			
	0	La	ke	Or	1 5	Sit	e																		() F	Reg	gul	.at	or	У	Ma	ąр												
	0	La	ke	Of	f	Si	.t	е																	() [De]	lin	ıea	.te	- ed	by	- Y (Coi	nsı	ul'	taı	nt							
	0	Ot:	he:	r I	'YE	pe	0	n S	Si	te															() [De]	Lin	ıea	.te	ed	by	y i	Arı	ny	C	orj	ps	0	f	En	gi	nee	ers	3
	0	Ot	he:	r I	УЕ	ре	0	££	S	it	е		I) C])th	ner	. (ic	ler	nt:	ify	y)					I	I				/	
10	ο.																					sti 15-				ee	n	id	en	ti	fi	eċ	l a	ıs	a			0	Ye	s	(И С	o		
11	1.			t] pei													ne	e (of	tł	ne	Wa	te	ers	he	ds	i	.de	nt	if	ie	d	ir	1				0	Ye	s	() N	О		
12	2.		ar wa		s a	as: ?	SO	ci	at	ed	. W	/it	h	A	Ā							wat ssi																0	Ye	s	(n C	o		
13	3.		ex id	is en	ti:	ng fi	i ed	mp a	er s	vi an	ou E	ıs L	c r	ove F	er o	ar n t	nd :he	wł e (ne: USI	re DA	t S	rb he oil tur	Sc S	oil Sur	. S	10	pe			se	i	ន						0	Ye	s	() n	O		
14	4.																					. a 0 f				ia	ce	nt										0	Ye	·s	() N	o.		_

15.	Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, culverts, etc)? Output Output Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, culverts, etc)?	
16.	What is the name of the municipality/entity that owns the separate storm sewer system?	
		_
17.	Does any runoff from the site enter a sewer classified as a Combined Sewer?	
18.	Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law?	
19.	Is this property owned by a state authority, state agency, federal government or local government?	
20.	Is this a remediation project being done under a Department approved work plan? (i.e. CERCLA, RCRA, Voluntary Cleanup Yes O No Agreement, etc.)	
21.	Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Yes O No Standards and Specifications for Erosion and Sediment Control (aka Blue Book)?	
22.	Does this construction activity require the development of a SWPPP that includes the post-construction stormwater management practice component (i.e. Runoff Reduction, Water Quality and Quantity Control practices/techniques)? If No, skip questions 23 and 27-39.	
23.	Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the current NYS O Yes O No Stormwater Management Design Manual?	

		_														_											_									
24	•	The	St	orn	nwa	ter	: P	ΟΙΙ	ut	ion	Pı	cev	en	tic	on	PΤ	an	(S	SWP	PP) v	ıas	p:	rer	par	ed	. b	у:								
(O Pr	ofe	ssi	ona	al	Eng	gin	eer	: (P.E	.)																									
(O Sc	il a	and	Wa	ate	r	Con	ser	rva	tio	n 1	Dis	tr	ic	t ((SW	CD)																		
(○ Re	gis	ter	ed	La	nds	sca	pe	Ar	chi	te	ct	(R	.L	. A)																				
(○ Ce	rti	fie	d 1	Pro	fe	ssi	ona.	al	in	Er	osi	.on	aı	nd	Se	di	mei	nt	Co	nt:	ro]	. (СP	ES	C)										
(Ow	ner	σ0\	era	ato	r																														
	O Ot		•																																	
			Т	Т	Τ	Τ	Τ				T	T	Т	T	T	T	Τ		Τ			Т	T	Т	T	T	Т	T		T	Т	T	T			
CIMDD	.D. D-			_																																
SWPP	PPI	ера	l ei			Т	Т	Т	Т		Τ	Ι									Π				Π			Τ	Т	Τ	Π		П	Т	Т	٦
			Ш,																														\sqcup			
Cont	act	Nam	ie (La	st,	, S	pac	ce,	Fi	irst	<u>;) </u>	Π	Π	Г		Π	Г		Г		Г				Г		Г	Τ	Т	Т	Π		П		\top	\neg
Ш																																				Ш
Mail	ing	Add	res	ss			1		_	1	1	1	ı				ı	l .	ı		ı						Ι		1		1		П			-
																																	Ш			╛
City			Т								T	Τ	I				Ι										Ι		T	T	Τ					_
																																	Ш			
Stat	.e 2	Zip				Г				_																										
			L			-[\perp																												
Phon	e								_									Faz	ĸ		ı				ſ					1						
		-			-																-				-											
Emai	1			1																		ı														_
									T																											
																											_		-							$-\!\!\!\!/$
_																																				

SWPPP Preparer Certification

I hereby certify that the Stormwater Pollution Prevention Plan (SWPPP) for this project has been prepared in accordance with the terms and conditions of the GP-0-15-002. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of this permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

First	Nan	ne .									MΙ						
Last 1	Name	2															
Sig	natı	ıre	,							1							
											Dat	te					
													/		/		

25.	Has a construction sequence schedule for the practices been prepared?	ne planned management O Yes O No					
26.	Select all of the erosion and sediment contemployed on the project site:	trol practices that will be					
	Temporary Structural	Vegetative Measures					
	Ocheck Dams	OBrush Matting					
	\bigcirc Construction Road Stabilization	O Dune Stabilization					
	O Dust Control	○ Grassed Waterway					
	○ Earth Dike	○ Mulching					
	○ Level Spreader	\bigcirc Protecting Vegetation					
	O Perimeter Dike/Swale	O Recreation Area Improvement					
	\bigcirc Pipe Slope Drain	○ Seeding					
	\bigcirc Portable Sediment Tank	○ Sodding					
	O Rock Dam	○ Straw/Hay Bale Dike					
	○ Sediment Basin	O Streambank Protection					
	○ Sediment Traps	○ Temporary Swale					
	○ Silt Fence	O Topsoiling					
	\bigcirc Stabilized Construction Entrance	O Vegetating Waterways					
	\bigcirc Storm Drain Inlet Protection	Permanent Structural					
	\bigcirc Straw/Hay Bale Dike						
	\bigcirc Temporary Access Waterway Crossing	O Debris Basin					
	\bigcirc Temporary Stormdrain Diversion	O Diversion					
	○ Temporary Swale	○ Grade Stabilization Structure					
	○ Turbidity Curtain	○ Land Grading					
	○ Water bars	○ Lined Waterway (Rock)					
		O Paved Channel (Concrete)					
	Biotechnical Property of the Britannian Property	O Paved Flume					
	OBrush Matting	○ Retaining Wall					
	○ Wattling	O Riprap Slope Protection					
		O Rock Outlet Protection					
Oth	<u>ner</u>	O Streambank Protection					

Post-construction Stormwater Management Practice (SMP) Requirements

Important: Completion of Questions 27-39 is not required
 if response to Question 22 is No.

- 27. Identify all site planning practices that were used to prepare the final site plan/layout for the project.
 - O Preservation of Undisturbed Areas
 - O Preservation of Buffers
 - O Reduction of Clearing and Grading
 - O Locating Development in Less Sensitive Areas
 - O Roadway Reduction
 - O Sidewalk Reduction
 - O Driveway Reduction
 - O Cul-de-sac Reduction
 - O Building Footprint Reduction
 - O Parking Reduction
- 27a. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6("Soil Restoration") of the Design Manual (2010 version).
 - O All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual (see page 5-22).
 - O Compacted areas were considered as impervious cover when calculating the **WQv Required**, and the compacted areas were assigned a post-construction Hydrologic Soil Group (HSG) designation that is one level less permeable than existing conditions for the hydrology analysis.
- 28. Provide the total Water Quality Volume (WQv) required for this project (based on final site plan/layout).

Total	$\mathbf{W}\mathbf{Q}\mathbf{v}$	Requ	ired	
			a	cre-feet

29. Identify the RR techniques (Area Reduction), RR techniques(Volume Reduction) and Standard SMPs with RRv Capacity in Table 1 (See Page 9) that were used to reduce the Total WQv Required(#28).

Also, provide in Table 1 the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

Note: Redevelopment projects shall use Tables 1 and 2 to identify the SMPs used to treat and/or reduce the WQv required. If runoff reduction techniques will not be used to reduce the required WQv, skip to question 33a after identifying the SMPs.

Table 1 - Runoff Reduction (RR) Techniques and Standard Stormwater Management Practices (SMPs)

	Total Contributing		Total Co	ntrı	buting
RR Techniques (Area Reduction)	Area (acres)	Im	pervious	Are	a(acres)
○ Conservation of Natural Areas (RR-1)		and/or].	
O Sheetflow to Riparian Buffers/Filters Strips (RR-2)		and/or			
○ Tree Planting/Tree Pit (RR-3)	•	and/or]•	
O Disconnection of Rooftop Runoff (RR-4)	, •	and/or		J•L	
RR Techniques (Volume Reduction)				1	
○ Vegetated Swale (RR-5) ······	• • • • • • • • • • • • • • • • • • • •	• • • • •		┦╹	
○ Rain Garden (RR-6) ······	• • • • • • • • • • • • • • • • • • • •	• • • • •		┤• ├─	
○ Stormwater Planter (RR-7)	• • • • • • • • • • • • • • • • • • • •	• • • • •		┤∙ ├─	
○ Rain Barrel/Cistern (RR-8)		• • • • •		 •	
○ Porous Pavement (RR-9)		• • • • •		_ •	
○ Green Roof (RR-10)	• • • • • • • • • • • • • • • • • • • •			J.	
Standard SMPs with RRv Capacity				- —	
O Infiltration Trench (I-1) ·····				 -	
O Infiltration Basin (I-2) ······	• • • • • • • • • • • • • • • • • • • •			 •	
Opry Well (I-3)	• • • • • • • • • • • • • • • • • • • •]-	
O Underground Infiltration System (I-4)	• • • • • • • • • • • • • • • • • • • •] -	
O Bioretention (F-5) ······]	
O Dry Swale (0-1)				J.	
Standard SMPs				1	
○ Micropool Extended Detention (P-1)	• • • • • • • • • • • • • • • • • • • •			- -	
○ Wet Pond (P-2) · · · · · · · · · · · · · · · · · · ·		• • • • •		- -	
○ Wet Extended Detention (P-3) ······		• • • • •		↓•	
○ Multiple Pond System (P-4) ······		• • • •		↓•	
O Pocket Pond (P-5) ······	• • • • • • • • • • • • • • • • • • • •	• • • • •]•	
○ Surface Sand Filter (F-1) ······	• • • • • • • • • • • • • • • • • • • •	• • • • •		 -	
○ Underground Sand Filter (F-2) ······	• • • • • • • • • • • • • • • • • • • •]-	
O Perimeter Sand Filter (F-3) ······	• • • • • • • • • • • • • • • • • • • •] . [
Organic Filter (F-4)		• • • •		-	
○ Shallow Wetland (W-1)	• • • • • • • • • • • • • • • • • • • •				
○ Extended Detention Wetland (W-2)					
O Pond/Wetland System (W-3)				 	
O Pocket Wetland (W-4)				1.	
○ Wet Swale (0-2)				1.	

Table 2 -Alternative SMPs (DO NOT INCLUDE PRACTICES BEING USED FOR PRETREATMENT ONLY) Total Contributing Alternative SMP Impervious Area(acres) ○ Hydrodynamic \bigcirc Wet Vault O Media Filter Other Provide the name and manufacturer of the Alternative SMPs (i.e. proprietary practice(s)) being used for WQv treatment. Name Manufacturer Note: Redevelopment projects which do not use RR techniques, shall use questions 28, 29, 33 and 33a to provide SMPs used, total WQv required and total WQv provided for the project. 30. Indicate the Total RRv provided by the RR techniques (Area/Volume Reduction) and Standard SMPs with RRv capacity identified in question 29. Total RRv provided acre-feet 31. Is the Total RRv provided (#30) greater than or equal to the total WQv required (#28). O Yes O No If Yes, go to question 36. If No, go to question 32. 32. Provide the Minimum RRv required based on HSG. [Minimum RRv Required = (P)(0.95)(Ai)/12, Ai=(S)(Aic)] Minimum RRv Required acre-feet 32a. Is the Total RRv provided (#30) greater than or equal to the O Yes O No Minimum RRv Required (#32)? If Yes, go to question 33. Note: Use the space provided in question #39 to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). A detailed evaluation of the specific site limitations and justification for not reducing 100% of the WQv required (#28) must also be included in the If No, sizing criteria has not been met, so NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

Identify the Standard SMPs in Table 1 and, if applicable, the Alternative SMPs in Table 2 that were used to treat the remaining total WQv(=Total WQv Required in 28 - Total RRv Provided in 30). Also, provide in Table 1 and 2 the total impervious area that contributes runoff to each practice selected. Note: Use Tables 1 and 2 to identify the SMPs used on Redevelopment projects. 33a. Indicate the Total WQv provided (i.e. WQv treated) by the SMPs identified in question #33 and Standard SMPs with RRv Capacity identified in question 29. WQv Provided acre-feet Note: For the standard SMPs with RRv capacity, the WQv provided by each practice = the WQv calculated using the contributing drainage area to the practice - RRv provided by the practice. (See Table 3.5 in Design Manual) 34. Provide the sum of the Total RRv provided (#30) and the WQv provided (#33a). Is the sum of the RRv provided (#30) and the WQv provided 35. (#33a) greater than or equal to the total WQv required (#28)? O Yes O No If Yes, go to question 36. If No, sizing criteria has not been met, so NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria. Provide the total Channel Protection Storage Volume (CPv) required and 36. provided or select waiver (36a), if applicable. CPv Required CPv Provided acre-feet acre-feet 36a. The need to provide channel protection has been waived because: O Site discharges directly to tidal waters or a fifth order or larger stream. O Reduction of the total CPv is achieved on site through runoff reduction techniques or infiltration systems. 37. Provide the Overbank Flood (Qp) and Extreme Flood (Qf) control criteria or select waiver (37a), if applicable. Total Overbank Flood Control Criteria (Qp) Pre-Development Post-development CFS CFS Total Extreme Flood Control Criteria (Qf)

Pre-Development

CFS

Post-development

CFS

	The need to meet the Qp and Qf criteria has been waived because:					
	O Site discharges directly to tidal waters					
	or a fifth order or larger stream.					
	○ Downstream analysis reveals that the Qp and Qf controls are not required					
	concroib are not required					
38.	Has a long term Operation and Maintenance Plan for the post-construction stormwater management practice(s) been Yes ONO					
	developed?					
	If Yes, Identify the entity responsible for the long term					
	Operation and Maintenance					
39.	Use this space to summarize the specific site limitations and justification					
	for not reducing 100% of WQv required(#28). (See question 32a)					
	This space can also be used for other pertinent project information.					

4285089826

40.	Identify other DEC permits, existing and new, that are required for this project/facility.
	O Air Pollution Control
	○ Coastal Erosion
	○ Hazardous Waste
	○ Long Island Wells
	○ Mined Land Reclamation
	○ Solid Waste
	O Navigable Waters Protection / Article 15
	O Water Quality Certificate
	○ Dam Safety
	○ Water Supply
	○ Freshwater Wetlands/Article 24
	○ Tidal Wetlands
	○ Wild, Scenic and Recreational Rivers
	O Stream Bed or Bank Protection / Article 15
	○ Endangered or Threatened Species(Incidental Take Permit)
	○ Individual SPDES
	○ SPDES Multi-Sector GP
	Other
	O None
41.	Does this project require a US Army Corps of Engineers Wetland Permit? If Yes, Indicate Size of Impact. O Yes O No
42.	Is this project subject to the requirements of a regulated, traditional land use control MS4? (If No, skip question 43)
43.	Has the "MS4 SWPPP Acceptance" form been signed by the principal executive officer or ranking elected official and submitted along with this NOI? \cite{NOI}
44.	If this NOI is being submitted for the purpose of continuing or transferring coverage under a general permit for stormwater runoff from construction

activities, please indicate the former SPDES number assigned.

Owner/Operator Certification

I have read or been advised of the permit conditions and believe that I understand them. I also understand that, under the terms of the permit, there may be reporting requirements. I hereby certify that this document and the corresponding documents were prepared under my direction or supervision. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. I further understand that coverage under the general permit will be identified in the acknowledgment that I will receive as a result of submitting this NOI and can be as long as sixty (60) business days as provided for in the general permit. I also understand that, by submitting this NOI, I am acknowledging that the SWPPP has been developed and will be implemented as the first element of construction, and agreeing to comply with all the terms and conditions of the general permit for which this NOI is being submitted.

MI
7
B. C.
Date

2.) NYSDEC Contractor Certification Statement

CONTRACTOR and SUBCONTRACTOR CERTIFICATION STATEMENT

for the New York State Department of Environmental Conservation (DEC) State Pollutant Discharge Elimination System Permit for Stormwater Discharges from Construction Activity (GP-0-15-002)

As per *Part III.A.5* on page 19 of *GP-0-15-002* (effective January 29, 2015):

'Prior to the *commencement of construction activity*, the *owner or operator* must identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting and maintaining the erosion and sediment control practices included in the SWPPP; and the contractor(s) and subcontractor(s) that will be responsible for constructing the post-construction stormwater management practices included in the SWPPP. The *owner or operator* shall have each of the contractors and subcontractors identify at least one person from their company that will be responsible for implementation of the SWPPP. This person shall be known as the *trained contractor*. The *owner or operator* shall ensure that at least one *trained contractor* is on site on a daily basis when soil disturbance activities are being performed.'

The owner or operator shall have each contractor and subcontractor involved in soil disturbance sign a copy of the following certification statement before they commence any construction activity:

Name of Construction Site	NYR DEC Permit ID	Municipality (MS4)
"I hereby certify under penalty of law to conditions of the SWPPP and agree to in inspector during a site inspection. I also us terms and conditions of the most current ve System ("SPDES") general permit for storm unlawful for any person to cause or contrib tan aware that there are significant penaltic true, including the possibility of	nplement any corrective of the conderstand that the owner is in the New York Standard from the New York Standard from the total and the submittion of wate its for submitting false information.	actions identified by the qualified or operator must comply with the ate Pollutant Discharge Elimination construction activities and that it is requality standards. Furthermore, formation, that I do not believe to be
Responsible Corporate Officer/Partner	Signature Date	
Name of above Signatory	Name of 0	Company
Title of above Signatory	Mailing A	Address
Telephone of Company	City, State	e and Zip
Identify the specific elements of the SW	PPP the contractor or s	subcontractor is responsible for:
'TRAINED CONTRACTOR' FOR THE	CERTIFIED CONTRA	CTOR OR SURCONTRACTOR

3.) Narrative

STORMWATER POLLUTION PREVENTION PLAN SPCA of Westchester 590 North State Road Town of Ossining - New York

A. INTRODUCTION

This Stormwater Pollution Prevention Plan & Stormwater Analysis presents the proposed Best Management Practices (BMPs) to control erosion, sedimentation, and manage stormwater during the construction of a proposed animal rescue facility, with associated driveway, parking, walkways and landscaping, located at 590 North State Road (SBL 90.11-1-50) in the Town of Ossining, Westchester County, New York.

This Plan consists of this narrative and a plan set entitled: "SPCA of Westchester, 590 North State Road, Town of Ossining, Westchester County, New York", all as prepared by Hudson Engineering and Consulting, P.C., Elmsford, New York, last revised October 9, 2018. The design is in accordance with the Town of Ossining requirements. The plans have been prepared to meet the requirements of the New York State Department of Environmental Conservation (NYSDEC). Since the project proposes to disturb approximately 2.21-acres, this Plan is required by the New York State Department of Environmental Conservation (NYSDEC) pursuant to the Phase II regulations under General Permit GP-0-15-002.

B. METHODOLOGY

The stormwater analysis was developed utilizing the Soil Conservation Service (SCS) TR-20 methodologies (HydroCad®) to assist with the drainage analysis and design of the mitigating practice. The "Complex Number" (CN) value determination is based on soil type, vegetation and land use. See Soil Map & Report contained herein. The "Time of Concentration" (T_c) is determined by the time wise longest flow path within each watershed. The CN and T_c data is input into the computer model. The project site was modeled for the peak rates of runoff from the 1-, 10-25- and 100-year Type III – 24-hour extreme storm events in both the Pre- and Post- Developed Conditions.

This project involves modifications to an existing developed property; therefore, this will be classified as redevelopment per the NYSDEC Phase II regulations. According to Section 9.2.1 of the NYS Stormwater Design Manual for Redevelopment entitled 'Sizing Criteria', the project is required to treat the Water Quality Volume (WQv) from 25% of the existing impervious area, as well as 100% of all new proposed impervious areas.

Impervious area coverage for Water Quality was calculated as follows:

Impervious Coverage						
25% Existing	7,293					
Impervious Area	square feet					
100% New	24,086					
Impervious Area	square feet					
Total Impervious Area to be Treated	31,379 square feet					

The stormwater management design is based on the NYSDEC "New York State Stormwater Management Design Manual", latest edition and "Controlling Urban Runoff: A Practical Manual for Planning and Designing Urban BMP'S", by the Metropolitan Washington Council of Governments. Stormwater quality has been analyzed in accordance with the guidelines set forth in the New York State General Permit for Storm Water Discharge, GP-0-15-002.

C. LIST OF PERMITS

The following is a list of permits and approvals required for the project along with the status.

- Town of Ossining Building Permit Pending
- Town of Ossining Planning Board Approval Pending
- NYSDEC SPDES General Permit # GP-0-15-002. Pending

D. PRE-DESIGN INVESTIGATIVE ANALYSIS

As previously stated, due to the presence of high groundwater witnessed throughout the site, as well as the location of the project in relation to an existing Town regulated wetland onsite, it was determined that conventional infiltration practices could not be utilized in the stormwater design (i.e. infiltration chambers, infiltration basins, etc.). Therefore, no deep hole testing or percolation testing was performed.

E. PRE-DEVELOPED CONDITION

In the pre-developed condition, the site was modeled as two watersheds: Watershed 1 and Watershed 2.

Watershed 1 contains approximately 115,658-square feet of tributary area which includes 43,520-square feet of area in the form of woods, 34,751-square feet of area in the form of lawn and landscaping, 1,928-square feet of area in the form of an existing watercourse, with the remaining 35,459-square feet of area as

impervious in the form of buildings, driveways and parking areas. The weighted Complex Number (CN) value is calculated as 80 and the Time of Concentration (Tc) is calculated as 12.1 minutes. The runoff from this watershed flows overland in a westerly direction where it enters an existing watercourse along the western property boundary and is subsequently conveyed to an existing culvert located at the northwest corner of the property at Design Point 1 – DP-1.

Watershed 2 contains approximately 51,330-square feet of tributary area which includes 20,932-square feet of area in the form of woods, 13,276-square feet of area in the form of brush, 8,519-square feet of area in the form of lawn and landscaping, 368-square feet of area in the form of an existing watercourse, with the remaining 8,235-square feet of area as impervious in the form of buildings and walkways. The weighted Complex Number (CN) value is calculated as 68 and the Time of Concentration (Tc) is calculated as 16.0 minutes. The runoff from this watershed flows overland in a westerly direction where it enters an existing watercourse at the northeastern corner of the property at Design Point 2.

The peak rates of runoff were calculated to be as follows:

Pre-Developed Conditions					
	1 Year	10 Year	25 Year	100 Year	
	cfs	cfs	cfs	cfs	
DP-1	2.70	7.70	10.73	17.10	
DP-2	0.40	1.96	3.03	5.45	

F. POST-DEVELOPED CONDITION

In the post-developed condition, the site is modeled as four (4) watersheds: Watersheds 1, 1A, 1B and 2.

Watershed 1 consists of the majority of the front of the property, including the existing wetland area. This watershed contains 74,951 square feet of tributary area which includes 42,591-square feet of area in the form of woods, 26,283-square feet of area in the form of lawn and landscaping, 1,928-square feet of area in the form of an existing watercourse, with the remaining 4,149-square feet of area as impervious in the form of proposed walkways and driveways. The weighted Complex Number (CN) value is calculated as 74 and the Time of Concentration (Tc) is calculated as 18.50 minutes. The runoff from this watershed flows overland in a westerly direction where it enters an existing watercourse along the western property boundary and is subsequently conveyed to an existing culvert located at the northwest corner of the property.

Watershed 1A consists of the proposed parking lot and adjacent landscaped areas. This watershed contains 35,602-square feet of area which includes 8,335-square feet of area in the form of lawn and landscaping and 27,267-square feet of impervious area in the form of the proposed parking lot and front walkways. The

weighted Complex Number (CN) value is calculated as 90 and the Time of Concentration (Tc) is calculated as 5.6 minutes. The runoff from this watershed flows overland in a southwesterly direction, where it is captured via catch basin and conveyed to a proposed hydrodynamic separator, which has been sized to pre-treat the runoff prior to discharging to a proposed NYSDEC Organic Filter Practice (Type F-4). This practice has been designed to treat the entire water quality volume for the entire property, as well as bypass the flows for all stromger storm events up to and including the 100-year storm. From here the treated runoff is conveyed to a proposed manhole adjacent to the practice, where it meets with the runoff from Watershed 1B, and is subsequently conveyed to Design Point 1, where it meets with the runoff from Watershed 1.

Watershed 1B consists of 20,885-square feet of roof area for the proposed building. The weighted Complex Number (CN) value is calculated as 98 with a direct entry Time of Concentration (Tc) of 1.0 minute. The runoff is captured via a series of roof drain leaders and is conveyed to a proposed hydrodynamic separator, which has been sized to treat the runoff from the roof area. From here the runoff is conveyed to a proposed manhole adjacent to the Organic Filter practice, where it meets with the runoff from Watershed 1A, and is subsequently conveyed to Design Point 1, where it meets with the runoff from Watershed 1.

Watershed 2 consists of the rear yard of the property and contains approximately 35,547-square feet of tributary area which includes 13,317-square feet of area in the form of woods, 6,548-square feet of area in the form of brush, 10,764-square feet of area in the form of lawn and landscaping, 368-square feet of area in the form of an existing watercourse, with the remaining 4,550-square feet of area as impervious in the form of proposed and walkways and kennel areas. The weighted Complex Number (CN) value is calculated as 66 and the Time of Concentration (Tc) is calculated as 14.1 minutes. The runoff from this watershed flows overland in a westerly direction where it enters an existing watercourse at the northeastern corner of the property at Design Point 2.

The peak rates of runoff were calculated to be as follows:

Post-Developed Conditions						
	1 Year 10 Year		25 Year	100 Year		
	cfs	cfs	cfs	cfs		
DP-1	DP-1 1.85 7.01		9.64	15.91		
DP-2	0.23	1.29	2.05	3.77		

G. SUMMARY OF FLOWS AT DESIGN POINT

Design Point	STORM EVENT						
_	1-year	10-year	25-year	100-year			
DP-1							
• Pre-[cfs]	2.70	7.70	10.73	17.10			
• Post-[cfs]	1.85	7.01	9.64	15.91			
DP-2							
Pre-[cfs]	0.40	1.96	3.03	5.45			
• Post-[cfs]	0.23	1.29	2.05	3.77			

The flow rates of runoff for all storm events in the proposed condition are less than those in the existing condition.

H. WATER QUALITY VOLUME

As previously mentioned, this project includes the redevelopment of the existing property, as well as the construction of 24,086 square feet of additional impervious area. According to Section 9.2.1 of the NYS Stormwater Design Manual for Redevelopment entitled 'Sizing Criteria', the project is required to treat the Water Quality Volume (WQv) from 25% of the existing impervious area, as well as 100% of all new proposed impervious areas.

Impervious area coverage for Water Quality was calculated as follows:

Impervious Coverage			
25% Existing	7,293		
Impervious Area	square feet		
100% New	24,086		
Impervious Area	square feet		
Total Impervious	31,379		
Area to be Treated	square feet		

Overall Water Quality Volume:

The Water Quality Volume (WQv) calculations were performed for the overall property as follows:

$$P= 90\% \quad \text{Rainfall} \qquad 1.5 \text{ -inches}$$

$$A_i = \quad \text{Impervious Area} = 31,379 \text{ -square feet}$$

$$A_i = 0.7204 \text{ -acres}$$

$$A_t = \quad \text{Tributary Area} = 166,988 \text{ -square feet}$$

$$A_t = 3.8335 \text{ -acres}$$

$$I = \quad \% \quad \text{Impervious} = 18.79\%$$

$$R_v = \quad 0.05 + 0.009(I); \text{ where } I = \text{Percent Impervious written as a percent}$$

$$R_v = \quad 0.219 \quad \text{(0.20 minimum)}$$

$$R_v = \quad 0.219$$

$$WQ_v = \frac{\text{(P x R_v x A_t)}}{12} = 0.10500 \quad \text{acre-feet} = 4573.81 \quad \text{cubic feet}$$

$$Rainfall = \quad 1.41 \text{ -inches} \rightarrow \quad 0.10600 \text{ acre-feet} \quad \text{OKAY}$$

The proposed Organic Filter (Type F-4) has been sized to treat the entire required water quality volume. The entire volume pre-treated by a 4-foot Diameter First Defense Unit hydrodynamic device. The device has also been sized to bypass the 100-year storm event. Water Quality routing calculations are contained within Section 9 of this report.

Watershed 1B - Roof Area

The Water Quality Volume (WQv) calculations were performed for the proposed roof area as follows:

P=	90%	Rainfall	1.5	-inches			
$A_i =$	Impervious Area =		76,417	-square feet			
		$A_i =$	1.7543	-acres			
$A_t =$	Tributary Area =		140,723	-square feet			
		$A_t =$	3.2306	-acres			
l=	% Imperv	ious =	54.30%				
R _v =	0.05+0.009(I); where I = Percent Impervious written as a percent						
		R _v =	0.539	(0.20 minimum)			
		R _v =	0.539				
$WQ_v =$	$(P \times R_v \times A_t)$. =	0.21755	acre-feet =	9476 43	cubic feet
12		_	0.21700	4010 1001 -	3470.40	Cubic icct	
7.65%	of WQ $_{V}$ =	0.01664	acre-feet =	724.947	cubic feet		
			_			_	
		Rainfall =	0.44	-inches →	0.04400	acre-feet	OKAY

The entire required water quality volume is treated via a 4-foot Diameter First Defense Unit hydrodynamic device. The device has also been sized to bypass the 100-year storm event. Water Quality routing calculations are contained within Section 7 of this report.

I. NYSDEC TABLE 3.1 DESIGN REGULATIONS:

Each mitigation practice is contained in Table 3.1 of the NYSDEC design regulations and is discussed below.

- Preservation of Undisturbed Areas: Permanent conservation easements of undisturbed areas are not proposed for this site.
- Preservation of Buffers. See above.
- Reduction of Clearing and Grading: All construction is occurring in areas previously disturbed.
- Locating Development in Less Sensitive Areas: No development is planned within sensitive areas.
- Open Space Design: Not applicable to this application.
- Soil Restoration: As required, all disturbed soil areas will be "deep tilled" prior to the establishment of ground cover. Deep tilling restores the absorptive quality of the soil.
- Roadway Reduction: No roadways are being proposed as part of this application.
- Sidewalk Reduction: All sidewalks have been designed to the minimum extent possible in order meet the required pedestrian traffic on-site.
- Driveway Reduction: No excess pavement was incorporated into the layout.
- Cul-de-sac Reduction: No Cul-de-sacs are being proposed as part of this application.
- Building Footprint Reduction: The proposed building footprint is considered the minimum footprint desired for the required operations.
- Parking Reduction: Parking for the facility has been provided as required by the Town of Ossining.
- Conservation of Natural Areas: All existing wetland areas are to remain undisturbed and protected as required by the Town of Ossining.
- Sheet Flow to riparian buffers or filter strips: Sheet flow to for Watersheds
 1 and 2 does exist through riparian buffers. However, no credit was taken for this treatment in the design calculations.
- Vegetated Open Swale: An "O-Type Swale" is not applicable to this site.
- Tree Planting/Tree Boxes: Tree Planting/Tree Boxes could be incorporated in the design, however, the entire water quality volume is already being treated via an organic filter practice.
- Disconnection of Rooftop Runoff: Not applicable to this application.
- Stream Daylighting for Redevelopment Projects: Not applicable to this application.
- Rain Gardens: Rain Gardens could be incorporated in the design, however, the entire water quality volume is already being treated via an organic filter practice.
- Green Roof: Green roof technology is not a viable alternative for this application.
- Stormwater Planters: Stormwater Planters could be incorporated in the design, however, the entire water quality volume is already being treated via an organic filter practice.
- Rain tank/Cistern: Rain tanks/Cisterns could be incorporated if desired.

 Porous Pavement: Porous Pavement could be incorporated into the design, however, due to the presence of high groundwater and ledge rock onsite, the use of porous pavement may not be effective.

J. CONSTRUCTION PHASE

During the construction phase of the project, a sediment and erosion control plan shall be implemented in accordance with the New York State Department of Environmental Conservation's Best Management Practices (BMP). The primary goals of the sediment and erosion control plan are to prevent the tracking of dirt and mud onto adjacent roads, to prevent mud and silt from entering into existing and proposed drainage facilities, and to protect the receiving waters from contamination during the construction.

<u>During construction, the party responsible for implementing the temporary (during construction) Stormwater Management facilities Maintenance Program will be the site contractor.</u> Contact information will be filed with the Town.

A New York State Professional Engineer or Certified Professional In Erosion and Sediment Control (P.E. or CPESC) shall conduct an assessment of the site prior to the commencement of construction and certify in an inspection report that the appropriate erosion and sediment controls shown on the plan have been adequately installed and/or implemented to ensure overall preparedness of the site for construction. Following the commencement of construction, site inspections shall be conducted by the P.E. or CPESC at least every 7 calendar days and within 24 hours of the end of a storm event of 0.5 inches or greater.

During each inspection, the representative shall record the following:

- 1. On a site map, indicate the extent of all disturbed site areas and drainage pathways. Indicate site areas that are expected to undergo initial disturbance or significant site work within the next 14-day period;
- 2. Indicate on a site map all areas of the site that have undergone temporary or permanent stabilization;
- 3. Indicate all disturbed site areas that have not undergone active site work during the previous 14-day period;
- 4. Inspect all sediment control practices and record approximate degree of sediment accumulation as a percentage of the sediment storage volume;
- 5. Inspect all erosion and sediment control practices and record all maintenance requirements. Identify any evidence of rill or gully erosion occurring on slopes and any loss of stabilizing vegetation or seeding/mulching. Document any excessive deposition of sediment or ponding water along the barrier. Record the depth of sediment within containment structures and any erosion near outlet and overflow structures.

6. All identified deficiencies.

The construction manager shall maintain a record of all inspection reports in a site logbook. The site logbook shall be maintained on-site and be made available to the Town of Ossining and/or the NYSDEC. A summary of the site inspection activities shall be posted on a monthly basis in a public accessible location at the site.

The projects anticipated start date is April 2019 and the anticipated completed date is April 2020.

K. CONSTRUCTION SEQUENCING

The following erosion control schedule shall be utilized:

- 1. Install construction entrance to the development area.
- 2. Establish construction staging area.
- 3. Install tree protection on trees as noted on plans.
- 4. Selective vegetation removal for silt fence installation.
- 5. Install silt fence down slope of all areas to be disturbed as shown on the plan.
- 6. Remove trees where necessary (clear & grub) for the proposed construction.
- 7. Strip topsoil and stockpile at the locations specified on the plans (up gradient of erosion control measures). Temporarily stabilize topsoil stockpiles (hydroseed during May 1st through October 31st planting season or by covering with a tarpaulin(s) November 1st through april 30th. Install silt fence around toe of slope.
- 8. Demolish any existing site features and/or structures noted as being removed on the construction documents and dispose of off-site.
- 9. Rough grade site.
- 10. Install additional erosion and sediment controls as necessary
- 11. Excavate and construct foundation for new building.
- 12. Rough grade parking lot and install drain inlets, hydrodynamic separator and manholes, as well as all associated onsite piping from existing brook up to location of proposed organic filter and roof drain leader connection at building.

- 13. Construct building. Install and connect all roof drain leaders to previously installed stormwater piping.
- 14. Construct organic surface filter practice (do not allow site runoff to enter filter until the entire tributary area is completely stabilized and 80% vegetative cover has been established in all landscaped areas, as well as within the organic filter).
- 15. Install curbing and sub-base courses.
- 16. Install 4"-6" topsoil, fine grade, seed the entire project site and install landscape plantings. Spread salt hay over seeded areas.
- 17. Install bituminous concrete top course.
- 18. Clean pavement, drain lines, catch basins and water quality devices. Clean organic filter practice.
- 19. Remove all temporary soil erosion and sediment control measures after the site is stabilized with vegetation.
- * Soil erosion and sediment control maintenance must occur weekly and prior to and after every ½" or greater rainfall event.

L. EROSION AND SEDIMENT CONTROL COMPONENTS

The primary aim of the soil and sediment control measures is to reduce soil erosion from areas stripped of vegetation during and after construction and to prevent silt from reaching the off-site drainage structures and downstream properties. As outlined in the Construction Sequencing schedule, the Sediment and Erosion Control Components are an integral component of the construction sequencing and will be implemented to control sedimentation and re-establish vegetation as soon as practicable.

Planned erosion and sedimentation control practices during construction include the installation, inspection and maintenance of the inlet protection, soil stockpile areas, diversion swales, sediment traps and silt fencing. General land grading practices, including land stabilization and construction sequencing are also integrated into the Sediment and Erosion Control Plan. Dust control is not expected to be a problem due to the relatively limited area of exposure, the undisturbed perimeter of trees around the project area and the relatively short time of exposure. Should excessive dust be generated, it will be controlled by sprinkling.

All proposed soil erosion and sediment control practices have been designed in accordance with the following publications:

- New York State standards and Specifications for Urban Erosion and Sediment Control, latest edition.
- New York State General Permit for Stormwater Discharges, GP-0-15-002 (General permit).
- "Reducing the Impacts of Stormwater Runoff from New Development", as published by the New York State Department of Environmental Conservation (NYSDEC), second edition, April, 1993.

The proposed soil erosion and sediment control devices include the planned erosion control practices outlined below. Maintenance procedures for each erosion control practice have also been outlined below.

SILT FENCE

Silt fence (geo-textile filter cloth) shall be placed in locations depicted on the approved plans. The purpose of the silt fence is to reduce the velocity of sediment laden stormwater from small drainage areas and to intercept the transported sediment load. In general, silt fence shall be used at the toe of slopes or intermediately within slopes where obvious channel concentration of stormwater is not present.

Maintenance

Silt fencing shall be inspected at a minimum of once per week and prior to and within 48 hours following a rain event ½" or greater. Inspections shall include ensuring that the fence material is tightly secured to the woven wire and the wire is secured to the wood posts. In addition, overlapping filter fabric shall be secure and the fabric shall be maintained a minimum of six (6) inches below grade. In the event that any "bulges" develop in the fence, that section of fence shall be replaced within 48 hours with new fence section. Any sediment build-up against the fence shall be removed within 48 hours and deposited on-site a minimum of 100 feet outside of any wetland or watercourse.

INLET PROTECTION

After driveway catch basins and surface inlets have been installed, these drain inlets will receive stormwater from the driveway, Temporary Diversion Swales and surrounding overland watersheds. In order to protect the receiving waters from sedimentation, the contractor shall install ¾ inch stone aggregate around the perimeter of all catch basins and surface inlets as illustrated on the approved plans. This barrier will allow stormwater to be filtered prior to reaching the basin inlet grate.

Maintenance

The stone aggregate shall be inspected weekly prior to and within 48 hours following a rain event ½" or greater. Care shall be taken to ensure that all stone aggregate are properly located and secure and do not become displaced. The stone aggregate shall be inspected for accumulated sediments and any accumulated sediment shall be removed from the device and deposited not less than 100 feet from wetland or watercourse.

TREE PROTECTION

All significant trees to be preserved located within the limits of disturbance and on the perimeter of the disturbance limits shall be protected from harm by erecting a 3' high (minimum) snow fence completely surrounding the tree. Snow fence should extend to the drip-line of the tree to be preserved. Trees designated to be protected shall be identified during the staking of the limits of disturbance for each construction phase.

Maintenance

The snow fence shall be inspected daily to ensure that the perimeter of the fence remains at the drip-line of the tree to be preserved. Any damaged portions of the fence shall be repaired or replaced within 48 hours. Care shall also be taken to ensure that no construction equipment is driven or parked within the drip-line of the tree to be preserved.

SOIL/SHOT ROCK STOCKPILING

All soil and shot rock stripped from the construction area during grubbing and mass grading shall be stockpiled in locations approved by the Town's representative, but in no case shall they be placed within 100' of a wetland or watercourse. The stockpiled soils shall be re-used during finish-grading to provide a suitable growing medium for plant establishment. Soil stockpiles shall be protected from erosion by vegetating the stockpile with rapidly – germinating grass seed or covering the stockpile with tarpaulin and surrounding it with either silt fence.

Maintenance

Sediment controls (silt fence) surrounding the stockpiles shall be inspected according to the recommended maintenance outline above. All stockpiles shall be inspected for signs of erosion or problems with seed establishment weekly and prior to and within 48 hours following a rain event ½" or greater.

GENERAL LAND GRADING

The intent of the Erosion & Sediment Control Plan is to control disturbed areas such that soils are protected from erosion by temporary methods and,

ultimately, by permanent vegetation. Where practicable, all cut and fill slopes shall be kept to a maximum slope of 2:1. In the event that a slope must exceed a 2:1 slope, it will be stabilized with stone riprap. On fill slopes, all material will be placed in layers not to exceed 12 inches in depth and adequately compacted. Where practicable, diversion swales shall be constructed on the top of all fill embankments to divert any overland flows away from the fill slopes.

SURFACE STABILIZATION

All disturbed will be protected from erosion with the use of vegetative measures (i.e., grass seed mix, sod) hydromulch netting or hay. When activities temporarily cease during construction, soil stockpiles and exposed soil should be stabilized by seed, mulch or other appropriate measures as soon as possible, but in no case more than 14 days after construction activity has ceased. All seeded areas will be re-seeded areas as necessary and mulch according to the site plan to maintain a vigorous, dense vegetative cover,

Erosion control barriers consisting of silt fencing shall be placed around exposed areas during construction. Where exposed areas are immediately uphill from a wetland or watercourse, the erosion control barrier will consist of double rows of silt fencing. Any areas stripped of vegetation during construction will be vegetated and/or mulch as soon as possible, but in no case more than 14 days to prevent erosion of the exposed soils. And topsoil removed during construction will be temporarily stockpiled for future use in grading and landscaping.

As mentioned above, temporary vegetation will be established to protect exposed soil areas during construction. If growing conditions are not suitable for the temporary vegetation, mulch will be used to the satisfaction of the Commissioner of Public Works. Materials that may be used for mulching include straw, hay, salt hay, wood fiber, synthetic soil stabilizers, mulch netting, sod or hydromulch. In site areas where significant erosion potential exists (steep slopes) and where specifically directed by the Town's representative, Curlex Excelsior erosion control blankets (manufactured by American Excelsior, or approved equal) shall be installed. A permanent vegetative cover will be established upon completion of construction of those areas that have been brought to finish-grade and to remain undisturbed.

DEWATERING

Prevent surface water and subsurface or ground water from flowing into excavations and trenches. Pump out any accumulated water.

Do not allow water to accumulate in excavations or trenches. Remove water from all excavations immediately to prevent softening of foundation bottoms, undercutting footings, and soil changes detrimental to the stability of subgrades and foundations. Furnish and maintain pumps, sumps, suction and discharge

piping systems, and other system components necessary to convey the water away from the Site.

Convey water removed from excavations, and rain water, to collecting or runoff area. Cut and maintain temporary drainage ditches and provide other necessary diversions outside excavation limits for each structure. Do not use trench excavations as temporary drainage ditches.

Provide temporary controls to restrict the velocity of discharged water as necessary to prevent erosion and siltation of receiving areas.

M. CONSTRUCTION PRACTICES TO MINIMIZE STORMWATER CONTAMINATION

General:

Adequate measures shall be taken to minimize contaminant particles arising from the discharge of solid materials, including building materials, grading operations, and the reclamation and placement of pavement, during project construction, including but not limited to:

- Building materials, garbage, and debris shall be cleaned up daily and deposited into dumpsters, which will be periodically removed from the site and appropriately disposed of. All dumpsters and containers left on-site shall be covered and surrounded with silt fence in order to prevent contaminants from leaving the site. Silt fencing shall be inspected on a weekly basis.
- Dump trucks hauling material from the construction site will be covered with a tarpaulin.
- The paved street adjacent to the site entrance will be swept daily to remove excess mud, dirt, or rock tracked from the site.
- Petroleum products will be stored in tightly sealed containers that are clearly labeled.
- All vehicles on site will be monitored for leaks and receive regular preventive maintenance to reduce the chance of leakage.
- All spills will be cleaned up immediately upon discovery. Spills large enough to reach the storm system will be reported to the National Response Center at 1-800-424-8802.
- Materials and equipment necessary for spill cleanup will be kept in the temporary material storage trailer onsite. Equipment will include, but not be limited to, brooms, dust pans, mops, rags, gloves, goggles, kitty litter, sand, saw dust, and plastic and metal trash containers.

- All paint containers and curing compounds will be tightly sealed and stored when not required for use. Excess paint will not be discharged to the storm system, but will be properly disposed according to the manufacturer's instructions.
- Sanitary waste will be collected from portable units a minimum of two times a week to avoid overfilling. All sanitary waste units shall be surrounded by silt fence to prevent contaminants from leaving the site. Silt fencing shall be inspected on a weekly basis.
- Any asphalt substances used on-site will be applied according to the manufacturer's recommendation.
- Fertilizers will be stored in a covered shed and partially used bags will be transferred to a sealable bin to avoid spills and will be applied only in the minimum amounts recommended by the manufacturer and worked into the soil to limit exposure to stormwater.
- No disturbed area shall be left un-stabilized for longer than 14 days during the growing season.
- When erosion is likely to be a problem, grubbing operations shall be scheduled and performed such that grading operations and permanent erosion control features can follow within 24 hours thereafter.
- As work progresses, patch seeding shall be done as required on areas previously treated to maintain or establish protective cover.
- Drainage pipes and swales/ditches shall generally be constructed in a sequence from outlet to inlet in order to stabilize outlet areas and ditches before water is directed to the new installation or any portion thereof, unless conditions unique to the location warrant an alternative method.

Spill Control & Spill Response:

- For all hazardous materials stored on site, the manufacturer's recommended methods for spill clean up will be clearly posted. Site personnel will be made aware of the procedures, and the locations of the information and cleanup supplies.
- Appropriate cleanup materials and equipment will be maintained by the Contractor in the materials storage area on-site. As appropriate, equipment and materials may include items such as booms, dust pans, mops, rags, gloves, goggles, kitty litter, sand, sawdust, and plastic and metal trash containers specifically for clean up purposes.
- All spills will be cleaned immediately after discovery and the materials disposed of properly.

- The spill area will be kept well ventilated and personnel will wear appropriate protective clothing to prevent injury from contact with a hazardous substance.
- After a spill, a report will be prepared describing the spill, what caused it, and the cleanup measures taken. The spill prevention plan will be adjusted to include measures to prevent this type of spill from reoccurring, as well as clean up instructions in the event of reoccurrences.
- The Contractor's site superintendent, responsible for day-to-day operations, will be the spill prevention and cleanup coordinator. The Contractor is responsible for ensuring that the site superintendent has had appropriate training for hazardous materials handling, spill management, and cleanup.
- The Contractor's site superintendent will be notified immediately when a spill or the threat of a spill is observed. The superintendent will assess the situation and determine the appropriate response.
- If spills represent an imminent threat of escaping erosion and sediment controls and entering receiving waters, personnel will be directed to respond immediately to contain the release and notify the superintendent after the situation has been stabilized.
- Spill kits containing appropriate materials and equipment for spill response and cleanup will be maintained by the Contractor at the site.
- If oil sheen is observed on surface water, action will be taken immediately
 to remove the material causing the sheen. The Contractor will use
 appropriate materials to contain and absorb the spill. The source of the oil
 sheen will also be identified and removed or repaired as necessary to
 prevent further releases.
- If a spill occurs the superintendent or the superintendent's designee will be responsible for completing the spill reporting form and for reporting the spill to the contacts listed below.
- Personnel with primary responsibility for spill response and clean up will receive training by the Contractor's site superintendent or designee. The training must include identifying the location of the spill kits and other spill response equipment and the use of spill response materials.
- Spill response equipment will be inspected and maintained as necessary to replace any materials used in spill response activities.

Spill Control Notification:

 A reportable spill is a quantity of five (5) gallons or more or any spill of oil which: (1) violates water quality standards, (2) produces a "sheen" on a surface water, or (3) causes a sludge or emulsion. This spill must be reported immediately to the agencies listed below.

- Any spill of oil or hazardous substance to waters of the state must be reported immediately by telephone to the following agencies:
 - 911 Police, Fire and EMS
 - Town of Ossining Building Department 101 Route 9A Ossining, NY 10562 Phone: (914) 941-3199
 - Briarcliff Manor Fire Department
 1111 Pleasantville Road
 Briarcliff Manor, NY 10510
 Phone: (914) 834-0016
 - NYS Department of Environmental Conservation (NYSDEC)
 Spill Reporting Hotline
 (1800) 457–7362
 - National Response Center: (1800) 424-8802
 - Local Emergency Planning Committee (LEPC)
 Westchester County Office of Emergency Management
 200 Bradhurst Avenue
 Hawthorne, NY 10532
 (914) 864–5450
 - Westchester County Department of Health (WCDOH)
 Spill Reporting Hotline
 (914) 813-5000
 - U.S. Environmental Protection Agency (USEPA)
 EPCRA Information Hotline
 1(800) 535–0202
 - U.S. Department of Labor and Occupational Safety and Health Administration (OSHA)
 Tarrytown, NY
 (914) 524–7510

N. STORMWATER MANAGEMENT FACILITIES MAINTENANCE PROGRAM

The following maintenance plan has been developed to maintain the proper function of all drainage and erosion and sediment control facilities:

Erosion & Sediment Control Maintenance:

During the construction of the project, the site erosion and sediment control measures as well as basin embankments and outlet structures will be inspected by the project superintendent once a week and/or within 24 hours following a

rainstorm ½" or greater. Any repairs required shall be performed in a timely manner. All sediment removal and/or repairs will be followed within 24 hours by re-vegetation. Remove sediment and correct erosion by re-seed eroded areas and gullies within 7 days.

General Stormwater Facilities Maintenance (Storm Sewer, Catch Basins/Drain Inlets, Manholes, Pre-treatment Device and Organic Filter)

All stormwater facilities shall be inspected immediately after completion of construction, and then monthly for the first three (3) months following the completion of the Project. Within the first three (3) months, inspections shall immediately be performed following a large storm event (i.e. producing 1/2" (one-half inch) of rain or greater. Thereafter, these facilities shall be inspected as described as follows. Upon inspection, facilities shall be immediately maintained and/or cleaned as may be required. Any site areas exhibiting soil erosion of any kind shall be immediately restored and stabilized with vegetation, mulch or stone, depending on the area to be stabilized.

Upon each inspection, all visible debris including, but not limited to, twigs, leaf and forest litter shall be removed from the swales, overflow discharge points and frames and grates of drainage structures.

Sumps – Catch Basin/Drain Inlets and Drain Manholes

All catch basin/drain inlets and drain manholes with sumps have been designed to trap sediment prior to its transport to the infiltration practice and, ultimately, downstream. These sumps will require periodic inspection and maintenance to ensure that adequate depth is maintained within the sumps.

All sumps shall be inspected once per month for the first three (3) months (after drainage system has been put into service). Thereafter, all sumps shall be inspected every four (4) months. The Owner, or their duly authorized representative, shall take measurements of the sump depth.

If sediment has accumulated to 1/2 (one-half) the depth of the sump, all sediment shall be removed from the sump. Sediments can be removed with hand-labor or with a vacuum truck.

The use of road salt shall be minimized for maintenance of roadway and driveway areas.

Hydrodynamic Separator:

The hydrodynamic separator (First Defense Unit) shall be inspected every six (6) months (Spring and Fall) for excess sediment accumulation. During dry

weather conditions, accumulated sediments shall be vacuumed out when sediment has reached 1/2 (one-half) the capacity of the isolated sump, or when an appreciable level of hydrocarbons and trash has accumulated, whichever occurs first.

Upon completion of construction, the First Defense Unit should be inspected quarterly during the first year in order to develop an appropriate schedule of maintenance. When the sediment pile is within 30 to 36 inches of the water surface, the system should be maintained. A vacuum truck shall be used to remove the accumulated sediment and debris. Refer to manufacturer's literature for detailed maintenance instructions.

Organic Surface Filter:

The organic filter shall be maintained during dry weather conditions. Silt/sediment shall be removed from the filter bed when the accumulation exceeds on inch. When the filtering capacity of the filter diminished substantially (i.e., when water ponds on the surface of the filter bed for more than 48-hours), the top few inches of discolored material shall be removed and shall be replaced with fresh material. The removed sediments shall be disposed in an acceptable manner (i.e., landfill).

O. CONCLUSION:

The stormwater management plan proposed meets and exceeds all the requirements set forth by the Town of Ossining and the New York State Department of Environmental Conservation (NYSDEC) for redevelopment projects. Design modification requirements that may occur during the approval process, will be performed and submitted for review to the Town of Ossining.

4.) Extreme Precipitation Table

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing Yes

State New York

Location

Longitude 73.815 degrees West **Latitude** 41.172 degrees North

Elevation 0 feet

Date/Time Mon, 29 Jan 2018 15:05:03 -0500

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.34	0.52	0.64	0.84	1.05	1.30	1yr	0.90	1.24	1.50	1.84	2.26	2.78	3.16	1yr	2.46	3.04	3.54	4.24	4.88	1yr
2yr	0.41	0.63	0.78	1.02	1.28	1.59	2yr	1.11	1.49	1.83	2.25	2.78	3.41	3.84	2yr	3.02	3.69	4.26	5.02	5.70	2yr
5yr	0.47	0.73	0.92	1.23	1.57	1.98	5yr	1.35	1.83	2.29	2.84	3.51	4.31	4.88	5yr	3.81	4.69	5.45	6.29	7.06	5yr
10yr	0.52	0.82	1.04	1.41	1.83	2.34	10yr	1.58	2.15	2.71	3.38	4.18	5.14	5.86	10yr	4.55	5.63	6.56	7.46	8.31	10yr
25yr	0.60	0.96	1.22	1.69	2.25	2.91	25yr	1.94	2.65	3.39	4.26	5.28	6.49	7.46	25yr	5.75	7.17	8.41	9.35	10.32	25yr
50yr	0.68	1.09	1.40	1.96	2.63	3.44	50yr	2.27	3.11	4.03	5.07	6.30	7.76	8.97	50yr	6.87	8.62	10.15	11.09	12.16	50yr
100yr	0.77	1.24	1.60	2.27	3.09	4.07	100yr	2.67	3.65	4.78	6.04	7.53	9.29	10.79	100yr	8.22	10.37	12.26	13.16	14.32	100yr
200yr	0.86	1.41	1.83	2.63	3.63	4.82	200yr	3.13	4.28	5.68	7.21	9.01	11.12	12.98	200yr	9.84	12.48	14.81	15.63	16.89	200yr
500yr	1.03	1.69	2.21	3.21	4.51	6.03	500yr	3.89	5.30	7.14	9.11	11.41	14.13	16.59	500yr	12.51	15.95	19.03	19.61	21.00	500yr

Lower Confidence Limits

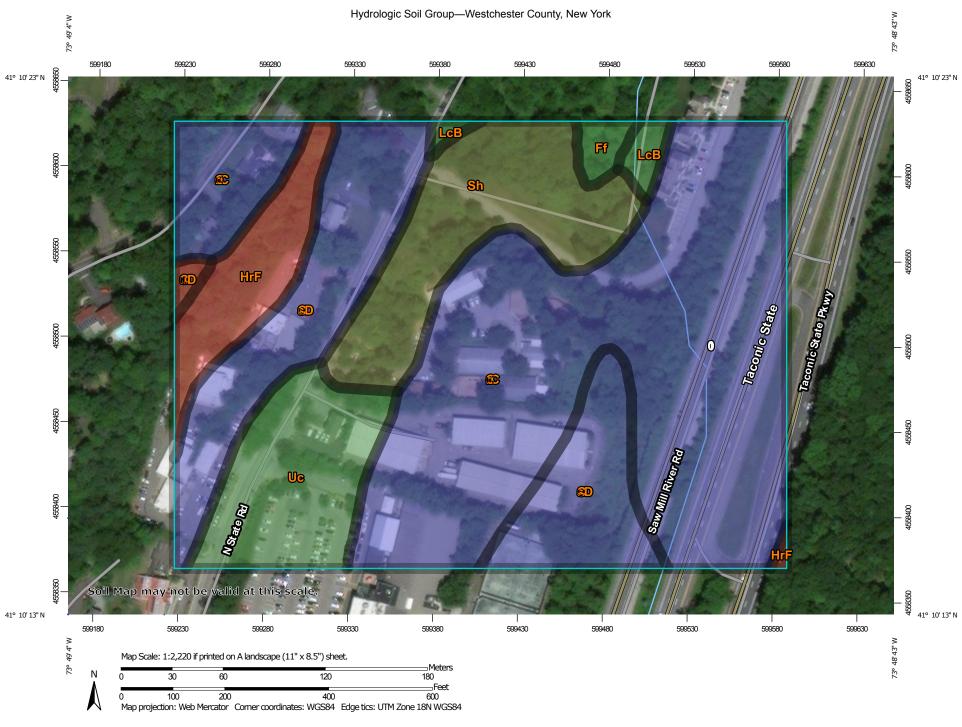
	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.43	0.52	0.70	0.86	1.15	1yr	0.75	1.12	1.37	1.70	2.14	2.42	2.91	1yr	2.14	2.80	3.28	3.77	4.34	1yr
2yr	0.39	0.60	0.74	1.00	1.23	1.48	2yr	1.06	1.44	1.70	2.16	2.70	3.29	3.71	2yr	2.92	3.56	4.10	4.82	5.50	2yr
5yr	0.43	0.67	0.83	1.14	1.45	1.73	5yr	1.25	1.69	1.98	2.51	3.15	4.05	4.48	5yr	3.58	4.31	4.97	5.80	6.49	5yr
10yr	0.47	0.73	0.90	1.26	1.63	1.93	10yr	1.41	1.89	2.23	2.79	3.54	4.54	5.18	10yr	4.02	4.98	5.74	6.45	7.02	10yr
25yr	0.53	0.81	1.00	1.43	1.89	2.23	25yr	1.63	2.18	2.59	3.18	4.15	5.47	6.29	25yr	4.84	6.05	7.23	7.60	7.91	25yr
50yr	0.58	0.88	1.10	1.58	2.12	2.49	50yr	1.83	2.44	2.93	3.52	4.68	6.32	7.29	50yr	5.60	7.01	8.44	8.60	8.60	50yr
100yr	0.64	0.96	1.21	1.74	2.39	2.79	100yr	2.07	2.73	3.31	3.88	5.28	7.33	8.47	100yr	6.49	8.15	9.86	9.73	9.31	100yr
200yr	0.70	1.05	1.33	1.93	2.69	3.12	200yr	2.32	3.05	3.75	4.29	5.97	8.53	9.87	200yr	7.55	9.49	11.54	10.98	10.05	200yr
500yr	0.80	1.19	1.53	2.22	3.15	3.62	500yr	2.72	3.54	4.43	4.88	7.05	10.44	12.09	500yr	9.24	11.63	14.22	12.91	11.01	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.38	0.58	0.71	0.96	1.18	1.42	1yr	1.02	1.39	1.62	2.12	2.54	3.00	3.40	1yr	2.65	3.27	3.85	4.67	5.33	1yr
2yr	0.42	0.65	0.80	1.08	1.33	1.60	2yr	1.15	1.56	1.82	2.36	2.91	3.55	4.00	2yr	3.15	3.84	4.42	5.26	6.04	2yr
5yr	0.51	0.78	0.97	1.33	1.70	2.01	5yr	1.47	1.97	2.33	3.03	3.80	4.60	5.30	5yr	4.07	5.10	5.94	6.76	7.57	5yr
10yr	0.60	0.92	1.14	1.60	2.06	2.41	10yr	1.78	2.36	2.81	3.70	4.65	5.79	6.59	10yr	5.12	6.34	7.44	8.52	9.42	10yr
25yr	0.75	1.14	1.42	2.03	2.67	3.08	25yr	2.30	3.01	3.62	4.85	6.07	7.63	8.78	25yr	6.75	8.45	9.64	11.27	12.25	25yr
50yr	0.89	1.35	1.68	2.42	3.26	3.72	50yr	2.81	3.64	4.36	5.96	7.43	9.41	10.90	50yr	8.32	10.48	11.99	13.93	14.96	50yr
100yr	1.07	1.61	2.02	2.92	4.00	4.50	100yr	3.45	4.40	5.28	7.35	9.08	11.59	13.54	100yr	10.26	13.02	14.92	17.22	18.27	100yr
200yr	1.27	1.92	2.43	3.52	4.91	5.43	200yr	4.23	5.31	6.38	9.05	11.10	14.28	16.79	200yr	12.64	16.14	18.55	21.27	22.35	200yr
500yr	1.64	2.44	3.13	4.55	6.47	6.98	500yr	5.59	6.82	8.19	11.98	14.49	18.80	22.32	500yr	16.64	21.46	24.75	28.20	29.23	500yr



5.) Soils Maps & Soils Data



MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:12.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D **Soil Rating Polygons** Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D contrasting soils that could have been shown at a more detailed Streams and Canals В Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: Westchester County, New York Survey Area Data: Version 13, Oct 8, 2017 C/D Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. D Not rated or not available Date(s) aerial images were photographed: Dec 31, 2009—Oct 5, 2016 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI						
CrC	Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky	В	11.6	49.3%						
CsD	Chatfield-Charlton complex, 15 to 35 percent slopes, very rocky	В	4.5	19.0%						
CuD	Chatfield-Hollis-Rock outcrop complex, 15 to 35 percent slopes	D	0.1	0.5%						
Ff	Fluvaquents-Udifluvents complex, frequently flooded	A/D	0.2	1.0%						
HrF	Hollis-Rock outcrop complex, 35 to 60 percent slopes	D	1.5	6.2%						
LcB	Leicester loam, 3 to 8 percent slopes, stony	A/D	0.3	1.2%						
Sh	Sun Ioam	C/D	3.0	12.9%						
Uc	Udorthents, wet substratum	A/D	2.3	9.9%						
Totals for Area of Inter	rest		23.5	100.0%						

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

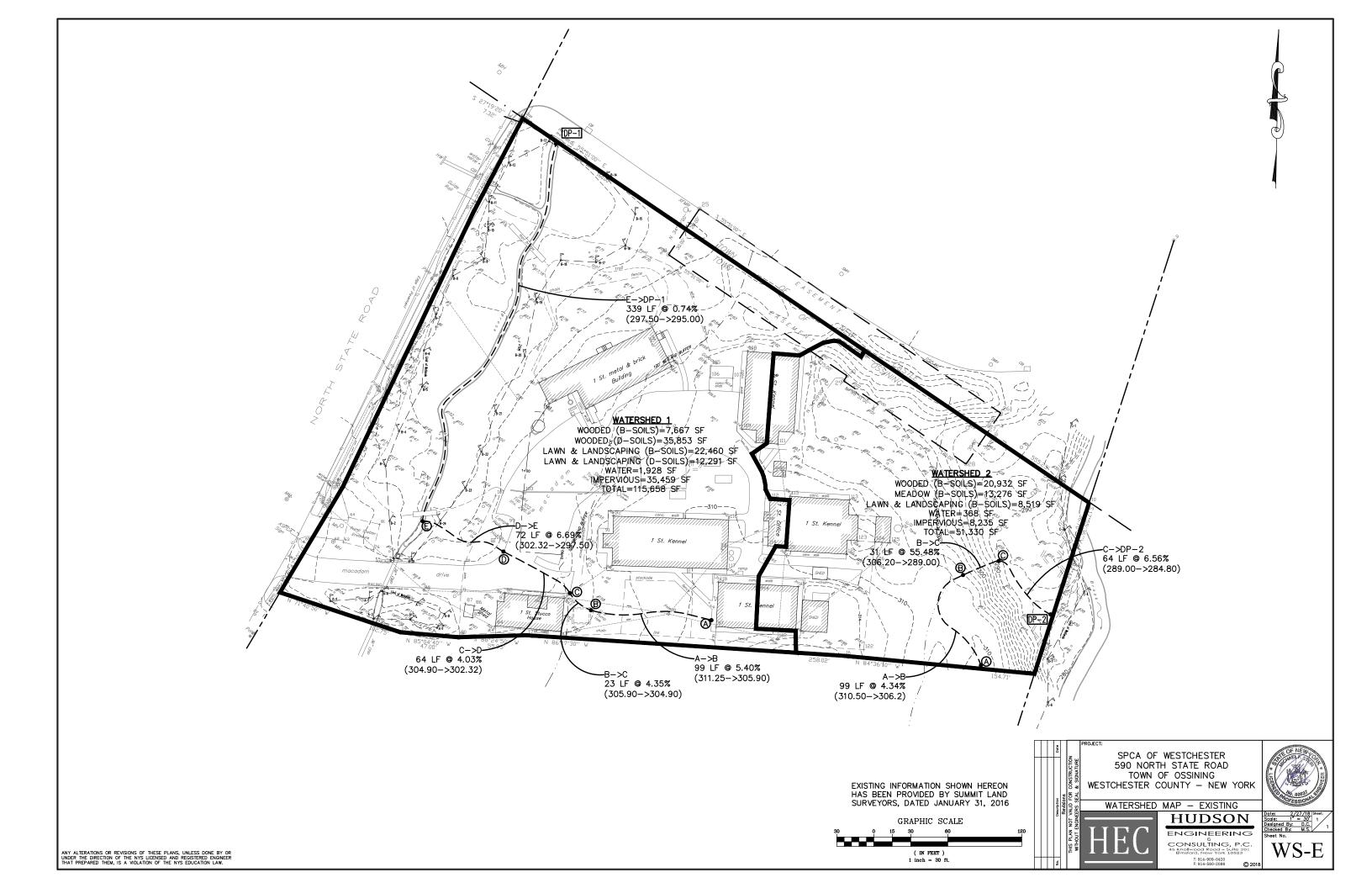
Rating Options

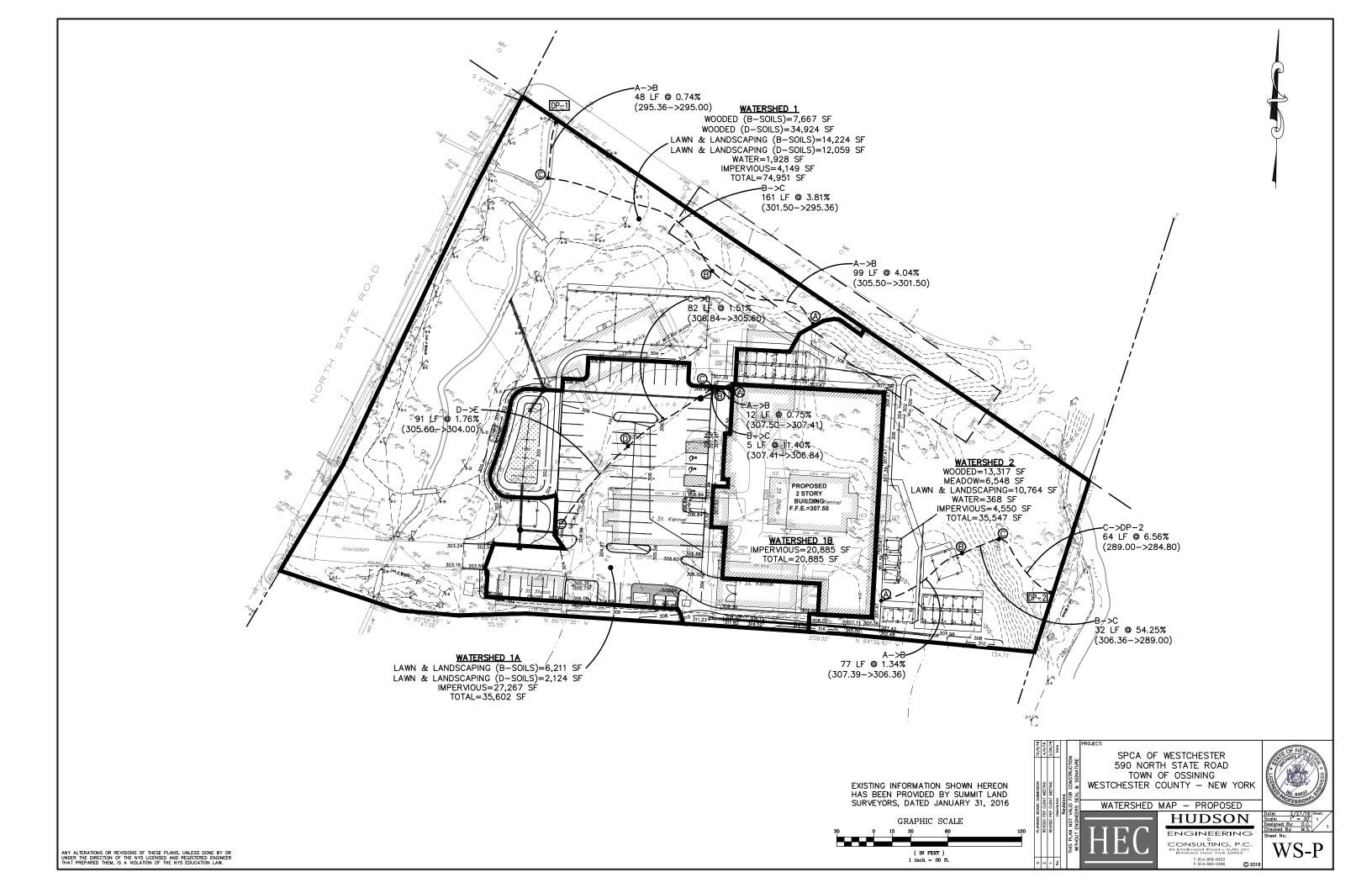
Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

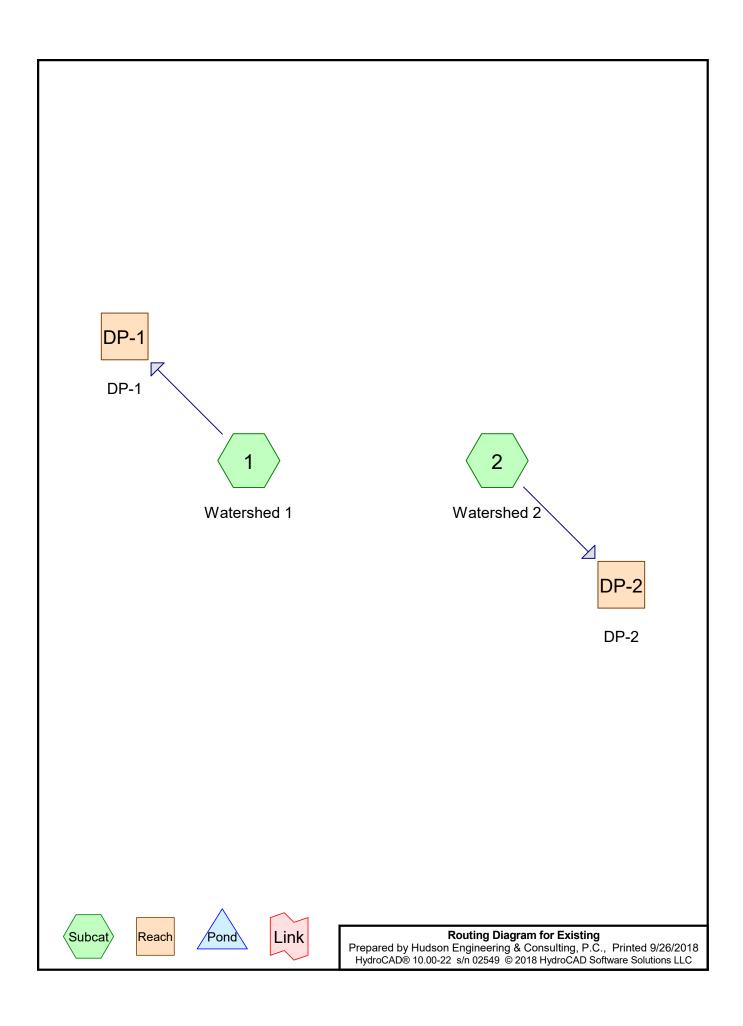
Tie-break Rule: Higher

	6.) Watershed Maps	
Hudson Engineering & Cor	nsulting, P.C.	





7.) Pre-Development Analysis of the 1-, 10-, 25- & 100-Year Storm Frequencies



Printed 9/26/2018 Page 2

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.711	61	>75% Grass cover, Good, HSG B (1, 2)
0.282	80	>75% Grass cover, Good, HSG D (1)
0.305	67	Brush, Poor, HSG B (2)
0.814	98	Parking Lot, Walkways, Buildings etc. (1)
0.189	98	Parking Lot, Walkways, Buildings, etc. (2)
0.008	98	Water Surface, HSG B (2)
0.044	98	Water Surface, HSG D (1)
0.176	55	Woods, Good, HSG B (1)
0.823	77	Woods, Good, HSG D (1)
0.481	58	Woods/grass comb., Good, HSG B (2)
3.834	76	TOTAL AREA

Existing

Prepared by Hudson Engineering & Consulting, P.C.

HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018 Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
1.681	HSG B	1, 2
0.000	HSG C	
1.149	HSG D	1
1.003	Other	1, 2
3.834		TOTAL AREA

Existing

Prepared by Hudson Engineering & Consulting, P.C.

HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018 Page 4

> Subcatcl Numbers

Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground
 (acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover
0.000	0.711	0.000	0.282	0.000	0.993	>75% Grass cover, Good
0.000	0.305	0.000	0.000	0.000	0.305	Brush, Poor
0.000	0.000	0.000	0.000	0.814	0.814	Parking Lot, Walkways, Buildings
						etc.
0.000	0.000	0.000	0.000	0.189	0.189	Parking Lot, Walkways, Buildings,
						etc.
0.000	0.008	0.000	0.044	0.000	0.053	Water Surface
0.000	0.176	0.000	0.823	0.000	0.999	Woods, Good
0.000	0.481	0.000	0.000	0.000	0.481	Woods/grass comb., Good
0.000	1.681	0.000	1.149	1.003	3.834	TOTAL AREA

Existing

Type III 24-hr 1-Year Rainfall=2.78"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC Printed 9/26/2018

Page 5

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Watershed 1 Runoff Area=115,658 sf 32.33% Impervious Runoff Depth=1.09"

Flow Length=598' Tc=12.1 min CN=80 Runoff=2.70 cfs 0.241 af

Subcatchment 2: Watershed 2 Runoff Area=51,330 sf 16.76% Impervious Runoff Depth=0.52"

Flow Length=194' Tc=16.0 min CN=68 Runoff=0.40 cfs 0.051 af

Reach DP-1: DP-1 Inflow=2.70 cfs 0.241 af

Outflow=2.70 cfs 0.241 af

Reach DP-2: DP-2 Inflow=0.40 cfs 0.051 af

Outflow=0.40 cfs 0.051 af

Total Runoff Area = 3.834 ac Runoff Volume = 0.291 af Average Runoff Depth = 0.91" 72.46% Pervious = 2.778 ac 27.54% Impervious = 1.056 ac

Summary for Subcatchment 1: Watershed 1

Runoff = 2.70 cfs @ 12.17 hrs, Volume= 0.241 af, Depth= 1.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Rainfall=2.78"

	Aı	rea (sf)	CN	Description			
		7,667	7 55 Woods, Good, HSG B				
	35,853 77 Woods, Good, HSG D						
	22,460 61 >75% Grass cover, Good, HSG B						
	12,291 80 >75% Grass cover, Good, HSG D						
		1,928		Water Surfa			
*		35,459	98	Parking Lot	, Walkways	s, Buildings etc.	
		15,658		Weighted A			
		78,271		67.67% Per			
		37,387		32.33% Imp	pervious Ar	ea	
	_		0.1			B 1.0	
	Tc	Length	Slope			Description	
	in)	(feet)	(ft/ft)		(cfs)		
ξ	9.2	99	0.0540	0.18		Sheet Flow, A->B	
						Grass: Dense n= 0.240 P2= 3.41"	
(0.3	23	0.0435	1.46		Shallow Concentrated Flow, B->C	
,		0.4	0.0400	4.00		Short Grass Pasture Kv= 7.0 fps	
(0.3	64	0.0403	4.08		Shallow Concentrated Flow, C->D	
,		70	0.0000	4.00		Paved Kv= 20.3 fps	
(0.9	73	0.0669	1.29		Shallow Concentrated Flow, D->E	
	1.4	339	0.0074	4.09	20.42	Woodland Kv= 5.0 fps Channel Flow, E->DP-1	
	1.4	339	0.0074	4.09	20.43	Area= 5.0 sf Perim= 7.0' r= 0.71'	
						n= 0.025 Earth, clean & winding	
	2 1	F00	Total			11- 0.020 Latti, olean & winding	
Ίz	2.1	598	Total				

Summary for Subcatchment 2: Watershed 2

Runoff = 0.40 cfs @ 12.27 hrs, Volume= 0.051 af, Depth= 0.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Rainfall=2.78"

	Area (sf)	CN	Description		
	20,932	58	Woods/grass comb., Good, HSG B		
	13,276	67	7 Brush, Poor, HSG B		
	8,519 61 >75% Grass cover, Good, HSG B				
	368	98	Water Surface, HSG B		
*	8,235	98	Parking Lot, Walkways, Buildings, etc.		
	51,330	68	Weighted Average		
	42,727		83.24% Pervious Area		
	8,603		16.76% Impervious Area		

Type III 24-hr 1-Year Rainfall=2.78"

Existing

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

<u> Page 7</u>

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	15.1	99	0.0434	0.11		Sheet Flow, A->B Woods: Light underbrush n= 0.400 P2= 3.41"
	0.1	31	0.5548	3.72		Shallow Concentrated Flow, B->C
	8.0	64	0.0656	1.28		Woodland Kv= 5.0 fps Shallow Concentrated Flow, C->DP-2 Woodland Kv= 5.0 fps
-	16.0	194	Total			<u> </u>

Summary for Reach DP-1: DP-1

Inflow Area = 2.655 ac, 32.33% Impervious, Inflow Depth = 1.09" for 1-Year event

Inflow = 2.70 cfs @ 12.17 hrs, Volume= 0.241 af

Outflow = 2.70 cfs @ 12.17 hrs, Volume= 0.241 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs

Summary for Reach DP-2: DP-2

Inflow Area = 1.178 ac, 16.76% Impervious, Inflow Depth = 0.52" for 1-Year event

Inflow = 0.40 cfs @ 12.27 hrs, Volume= 0.051 af

Outflow = 0.40 cfs @ 12.27 hrs, Volume= 0.051 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs

Existing

Type III 24-hr 10-Year Rainfall=5.14"

Prepared by Hudson Engineering & Consulting, P.C.

Printed 9/26/2018

HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Page 8

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Watershed 1 Runoff Area=115,658 sf 32.33% Impervious Runoff Depth=3.02"

Flow Length=598' Tc=12.1 min CN=80 Runoff=7.70 cfs 0.667 af

Subcatchment 2: Watershed 2 Runoff Area=51,330 sf 16.76% Impervious Runoff Depth=1.98"

Flow Length=194' Tc=16.0 min CN=68 Runoff=1.96 cfs 0.194 af

Reach DP-1: DP-1 Inflow=7.70 cfs 0.667 af

Outflow=7.70 cfs 0.667 af

Reach DP-2: DP-2 Inflow=1.96 cfs 0.194 af

Outflow=1.96 cfs 0.194 af

Total Runoff Area = 3.834 ac Runoff Volume = 0.862 af Average Runoff Depth = 2.70" 72.46% Pervious = 2.778 ac 27.54% Impervious = 1.056 ac

Existing

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 9

Summary for Subcatchment 1: Watershed 1

Runoff = 7.70 cfs @ 12.17 hrs, Volume= 0.667 af, Depth= 3.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.14"

A	rea (sf)	CN [Description				
	7,667 55 Woods, Good, HSG B 35,853 77 Woods, Good, HSG D						
	22,460	61 >	>75% Gras	s cover, Go	ood, HSG B		
	12,291	80 >	>75% Gras	s cover, Go	ood, HSG D		
	1,928	98 \	Nater Surfa	ace, HSG D			
*	35,459	98 F	Parking Lot	, Walkways	s, Buildings etc.		
1	15,658	80 V	Veighted A	verage			
	78,271	6	67.67% Per	vious Area			
	37,387	3	32.33% Imp	ervious Are	ea		
_							
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
9.2	99	0.0540	0.18		Sheet Flow, A->B		
					Grass: Dense n= 0.240 P2= 3.41"		
0.3	23	0.0435	1.46		Shallow Concentrated Flow, B->C		
					Short Grass Pasture Kv= 7.0 fps		
0.3	64	0.0403	4.08		Shallow Concentrated Flow, C->D		
0.0	70	0.0000	4.00		Paved Kv= 20.3 fps		
0.9	73	0.0669	1.29		Shallow Concentrated Flow, D->E		
4.4	220	0.0074	4.00	20.42	Woodland Kv= 5.0 fps		
1.4	339	0.0074	4.09	20.43	•		
					Area= 5.0 sf Perim= 7.0' r= 0.71'		
40.4		T-4-1			n= 0.025 Earth, clean & winding		
12.1	598	Total					

Summary for Subcatchment 2: Watershed 2

Runoff = 1.96 cfs @ 12.23 hrs, Volume= 0.194 af, Depth= 1.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.14"

	Area (sf)	CN	Description
	20,932	58	Woods/grass comb., Good, HSG B
	13,276	67	Brush, Poor, HSG B
	8,519	61	>75% Grass cover, Good, HSG B
	368	98	Water Surface, HSG B
*	8,235	98	Parking Lot, Walkways, Buildings, etc.
	51,330	68	Weighted Average
	42,727		83.24% Pervious Area
	8,603		16.76% Impervious Area

Type III 24-hr 10-Year Rainfall=5.14"

ng & Consulting, P.C.

Printed 9/26/2018

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Existing

Page 10

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	99	0.0434	0.11		Sheet Flow, A->B
0.1	31	0.5548	3.72		Woods: Light underbrush n= 0.400 P2= 3.41" Shallow Concentrated Flow, B->C
0.8	64	0.0656	1.28		Woodland Kv= 5.0 fps Shallow Concentrated Flow, C->DP-2
 0.6	04	0.0030	1.20		Woodland Kv= 5.0 fps
16.0	194	Total			

Summary for Reach DP-1: DP-1

Inflow Area = 2.655 ac, 32.33% Impervious, Inflow Depth = 3.02" for 10-Year event

Inflow = 7.70 cfs @ 12.17 hrs, Volume= 0.667 af

Outflow = 7.70 cfs @ 12.17 hrs, Volume= 0.667 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs

Summary for Reach DP-2: DP-2

Inflow Area = 1.178 ac, 16.76% Impervious, Inflow Depth = 1.98" for 10-Year event

Inflow = 1.96 cfs @ 12.23 hrs, Volume= 0.194 af

Outflow = 1.96 cfs @ 12.23 hrs, Volume= 0.194 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs

Existing

Type III 24-hr 25-Year Rainfall=6.49"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC Printed 9/26/2018

Page 11

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Watershed 1 Runoff Area=115,658 sf 32.33% Impervious Runoff Depth=4.23"

Flow Length=598' Tc=12.1 min CN=80 Runoff=10.73 cfs 0.935 af

Subcatchment 2: Watershed 2 Runoff Area=51,330 sf 16.76% Impervious Runoff Depth=3.00"

Flow Length=194' Tc=16.0 min CN=68 Runoff=3.03 cfs 0.295 af

Reach DP-1: DP-1 Inflow=10.73 cfs 0.935 af

Outflow=10.73 cfs 0.935 af

Reach DP-2: DP-2 Inflow=3.03 cfs 0.295 af

Outflow=3.03 cfs 0.295 af

Total Runoff Area = 3.834 ac Runoff Volume = 1.230 af Average Runoff Depth = 3.85" 72.46% Pervious = 2.778 ac 27.54% Impervious = 1.056 ac

Existing

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 12

Summary for Subcatchment 1: Watershed 1

Runoff = 10.73 cfs @ 12.17 hrs, Volume= 0.935 af, Depth= 4.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.49"

A	rea (sf)	CN [Description				
	7,667 55 Woods, Good, HSG B 35,853 77 Woods, Good, HSG D						
	22,460	61 >	>75% Gras	s cover, Go	ood, HSG B		
	12,291	80 >	>75% Gras	s cover, Go	ood, HSG D		
	1,928	98 \	Nater Surfa	ace, HSG D			
*	35,459	98 F	Parking Lot	, Walkways	s, Buildings etc.		
1	15,658	80 V	Veighted A	verage			
	78,271	6	67.67% Per	vious Area			
	37,387	3	32.33% Imp	ervious Are	ea		
_							
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
9.2	99	0.0540	0.18		Sheet Flow, A->B		
					Grass: Dense n= 0.240 P2= 3.41"		
0.3	23	0.0435	1.46		Shallow Concentrated Flow, B->C		
					Short Grass Pasture Kv= 7.0 fps		
0.3	64	0.0403	4.08		Shallow Concentrated Flow, C->D		
0.0	70	0.0000	4.00		Paved Kv= 20.3 fps		
0.9	73	0.0669	1.29		Shallow Concentrated Flow, D->E		
4.4	220	0.0074	4.00	20.42	Woodland Kv= 5.0 fps		
1.4	339	0.0074	4.09	20.43	•		
					Area= 5.0 sf Perim= 7.0' r= 0.71'		
40.4		T-4-1			n= 0.025 Earth, clean & winding		
12.1	598	Total					

Summary for Subcatchment 2: Watershed 2

Runoff = 3.03 cfs @ 12.22 hrs, Volume= 0.295 af, Depth= 3.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.49"

	Area (sf)	CN	Description
	20,932	58	Woods/grass comb., Good, HSG B
	13,276	67	Brush, Poor, HSG B
	8,519	61	>75% Grass cover, Good, HSG B
	368	98	Water Surface, HSG B
*	8,235	98	Parking Lot, Walkways, Buildings, etc.
	51,330	68	Weighted Average
	42,727		83.24% Pervious Area
	8,603		16.76% Impervious Area

Existing	Type III 24-hr	25-Year Rainfall=6.49"
Prepared by Hudson Engineering & Consulting, P.C.		Printed 9/26/2018

Woodland Kv= 5.0 fps

HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Capacity

(cfs)

Velocity

(ft/sec)

0.11

3.72

1.28

Slope

(ft/ft)

99 0.0434

31 0.5548

0.0656

O Software Solutions LLC	Page 13					
Description						
Description						
Sheet Flow, A->B						
Woods: Light underbrush n= 0.400 P2= 3.41"						
Shallow Concentrated Flow, B->C						
Woodland Kv= 5.0 fps						
Shallow Concentrated Flow, C->DP-2						

194 16.0 Total

Tc Length

(min)

15.1

0.1

8.0

(feet)

Summary for Reach DP-1: DP-1

2.655 ac, 32.33% Impervious, Inflow Depth = 4.23" for 25-Year event Inflow Area =

10.73 cfs @ 12.17 hrs, Volume= Inflow 0.935 af

10.73 cfs @ 12.17 hrs, Volume= Outflow 0.935 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs

Summary for Reach DP-2: DP-2

1.178 ac, 16.76% Impervious, Inflow Depth = 3.00" for 25-Year event Inflow Area =

Inflow 3.03 cfs @ 12.22 hrs, Volume= 0.295 af

Outflow 3.03 cfs @ 12.22 hrs, Volume= 0.295 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs

Existing

Type III 24-hr 100-Year Rainfall=9.29"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC Printed 9/26/2018

Page 14

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Watershed 1 Runoff Area=115,658 sf 32.33% Impervious Runoff Depth=6.84"

Flow Length=598' Tc=12.1 min CN=80 Runoff=17.10 cfs 1.514 af

Subcatchment 2: Watershed 2 Runoff Area=51,330 sf 16.76% Impervious Runoff Depth=5.34"

Flow Length=194' Tc=16.0 min CN=68 Runoff=5.45 cfs 0.524 af

Reach DP-1: DP-1 Inflow=17.10 cfs 1.514 af

Outflow=17.10 cfs 1.514 af

Reach DP-2: DP-2 Inflow=5.45 cfs 0.524 af

Outflow=5.45 cfs 0.524 af

Total Runoff Area = 3.834 ac Runoff Volume = 2.039 af Average Runoff Depth = 6.38" 72.46% Pervious = 2.778 ac 27.54% Impervious = 1.056 ac

Printed 9/26/2018

Page 15

Summary for Subcatchment 1: Watershed 1

Runoff = 17.10 cfs @ 12.16 hrs, Volume= 1.514 af, Depth= 6.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.29"

A	rea (sf)	CN E	escription				
7,667 55 Woods, Good, HSG B							
35,853 77 Woods, Good, HSG D							
	22,460				ood, HSG B		
	12,291				ood, HSG D		
_	1,928			ace, HSG D			
*	35,459				s, Buildings etc.		
1	15,658		Veighted A				
	78,271	_	-	vious Area			
	37,387	3	2.33% Imp	ervious Are	2 8		
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description		
9.2	99	0.0540	0.18	(010)	Sheet Flow, A->B		
5.2	33	0.0040	0.10		Grass: Dense n= 0.240 P2= 3.41"		
0.3	23	0.0435	1.46		Shallow Concentrated Flow, B->C		
					Short Grass Pasture Kv= 7.0 fps		
0.3	64	0.0403	4.08		Shallow Concentrated Flow, C->D		
					Paved Kv= 20.3 fps		
0.9	73	0.0669	1.29		Shallow Concentrated Flow, D->E		
					Woodland Kv= 5.0 fps		
1.4	339	0.0074	4.09	20.43	·		
					Area= 5.0 sf Perim= 7.0' r= 0.71'		
					n= 0.025 Earth, clean & winding		
12.1	598	Total					

Summary for Subcatchment 2: Watershed 2

Runoff = 5.45 cfs @ 12.22 hrs, Volume= 0.524 af, Depth= 5.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.29"

	Area (sf)	CN	Description
	20,932	58	Woods/grass comb., Good, HSG B
	13,276	67	Brush, Poor, HSG B
	8,519	61	>75% Grass cover, Good, HSG B
	368	98	Water Surface, HSG B
*	8,235	98	Parking Lot, Walkways, Buildings, etc.
	51,330	68	Weighted Average
	42,727		83.24% Pervious Area
	8,603		16.76% Impervious Area

Type III 24-hr 100-Year Rainfall=9.29" Printed 9/26/2018

ExistingPrepared by Hudson Engineering & Consulting, P.C.

HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Page 16

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	99	0.0434	0.11		Sheet Flow, A->B
0.1	31	0.5548	3.72		Woods: Light underbrush n= 0.400 P2= 3.41" Shallow Concentrated Flow, B->C
0.8	64	0.0656	1.28		Woodland Kv= 5.0 fps Shallow Concentrated Flow, C->DP-2
 0.6	04	0.0030	1.20		Woodland Kv= 5.0 fps
16.0	194	Total			

Summary for Reach DP-1: DP-1

Inflow Area = 2.655 ac, 32.33% Impervious, Inflow Depth = 6.84" for 100-Year event

Inflow = 17.10 cfs @ 12.16 hrs, Volume= 1.514 af

Outflow = 17.10 cfs @ 12.16 hrs, Volume= 1.514 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs

Summary for Reach DP-2: DP-2

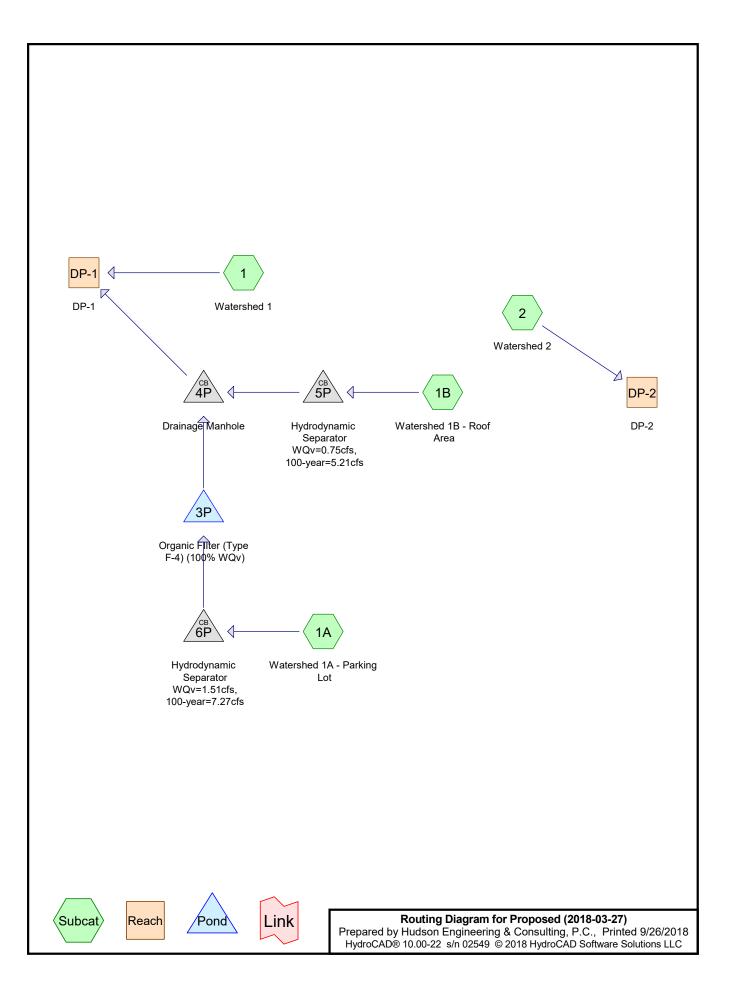
Inflow Area = 1.178 ac, 16.76% Impervious, Inflow Depth = 5.34" for 100-Year event

Inflow = 5.45 cfs @ 12.22 hrs, Volume= 0.524 af

Outflow = 5.45 cfs @ 12.22 hrs, Volume= 0.524 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs

8.) Post-Development Analysis of the 1-, 10-, 25- & 100-Year Storm Frequencies



Printed 9/26/2018 Page 2

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.716	61	>75% Grass cover, Good, HSG B (1, 1A, 2)
0.326	80	>75% Grass cover, Good, HSG D (1, 1A)
0.150	67	Brush, Poor, HSG B (2)
0.721	98	Parking Lot, Walkways, Buildings etc. (1, 1A)
0.104	98	Parking Lot, Walkways, Buildings, etc. (2)
0.479	98	Proposed Building (1B)
0.008	98	Water Surface, HSG B (2)
0.044	98	Water Surface, HSG D (1)
0.176	55	Woods, Good, HSG B (1)
0.802	77	Woods, Good, HSG D (1)
0.306	58	Woods/grass comb., Good, HSG B (2)
3.833	79	TOTAL AREA

Proposed (2018-03-27)
Prepared by Hudson Engineering & Consulting, P.C.
HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018 Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
1.357	HSG B	1, 1A, 2
0.000	HSG C	
1.172	HSG D	1, 1A
1.305	Other	1, 1A, 1B, 2
3.833		TOTAL AREA

Proposed (2018-03-27)
Prepared by Hudson Engineering & Consulting, P.C.
HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018 Page 4

> Subcatcl Numbers

Ground Covers (all nodes)

HSG	-A HSG	-B HSG-C	HSG-D	Other	Total	Ground
(acre	es) (acre	es) (acres)	(acres)	(acres)	(acres)	Cover
0.0	00 0.7	16 0.000	0.326	0.000	1.042	>75% Grass cover, Good
0.0	00 0.1	50 0.000	0.000	0.000	0.150	Brush, Poor
0.0	0.0	0.000	0.000	0.721	0.721	Parking Lot, Walkways, Buildings
						etc.
0.0	0.0	0.000	0.000	0.104	0.104	Parking Lot, Walkways, Buildings,
						etc.
0.0	0.0	0.000	0.000	0.479	0.479	Proposed Building
0.0	0.0	0.000	0.044	0.000	0.053	Water Surface
0.0	00 0.1	76 0.000	0.802	0.000	0.978	Woods, Good
0.0	00 0.3	0.000	0.000	0.000	0.306	Woods/grass comb., Good
0.0	00 1.3	57 0.000	1.172	1.305	3.833	TOTAL AREA

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 5

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Watershed 1 Runoff Area=74,951 sf 8.11% Impervious Runoff Depth=0.77"

Flow Length=308' Tc=18.5 min CN=74 Runoff=0.98 cfs 0.111 af

Subcatchment 1A: Watershed 1A - Parking Runoff Area=35,602 sf 76.59% Impervious Runoff Depth=1.78" Flow Length=190' Tc=5.6 min CN=90 Runoff=1.72 cfs 0.121 af

Subcatchment 1B: Watershed 1B - Roof Runoff Area=20,885 sf 100.00% Impervious Runoff Depth=2.55" Tc=1.0 min CN=98 Runoff=1.54 cfs 0.102 af

Subcatchment 2: Watershed 2 Runoff Area=35,547 sf 13.84% Impervious Runoff Depth=0.44" Flow Length=173' Tc=14.1 min CN=66 Runoff=0.23 cfs 0.030 af

Reach DP-1: DP-1 Inflow=1.85 cfs 0.334 af

Outflow=1.85 cfs 0.334 af

Reach DP-2: DP-2 Inflow=0.23 cfs 0.030 af

Outflow=0.23 cfs 0.030 af

Pond 3P: Organic Filter (Type F-4) (100% Peak Elev=301.62' Storage=2,737 cf Inflow=1.72 cfs 0.121 af

Outflow=0.13 cfs 0.121 af

Pond 4P: Drainage Manhole Peak Elev=296.90' Inflow=1.59 cfs 0.223 af

18.0" Round Culvert n=0.013 L=76.0' S=0.0099 '/' Outflow=1.59 cfs 0.223 af

Pond 5P: Hydrodynamic Separator WQv=0.75cfs, Peak Elev=304.24' Inflow=1.54 cfs 0.102 af

12.0" Round Culvert n=0.013 L=123.0' S=0.0587 '/' Outflow=1.54 cfs 0.102 af

Pond 6P: Hydrodynamic Separator WQv=1.51cfs, Peak Elev=301.62' Inflow=1.72 cfs 0.121 af

18.0" Round Culvert n=0.013 L=35.0' S=0.0200 '/' Outflow=1.72 cfs 0.121 af

Total Runoff Area = 3.833 ac Runoff Volume = 0.364 af Average Runoff Depth = 1.14" 64.58% Pervious = 2.476 ac 35.42% Impervious = 1.358 ac

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 6

Summary for Subcatchment 1: Watershed 1

Runoff = 0.98 cfs @ 12.28 hrs, Volume= 0.111 af, Depth= 0.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Rainfall=2.78"

	Α	rea (sf)	CN [Description						
		7,667	55 V	55 Woods, Good, HSG B						
		34,924	77 V	Voods, Go	od, HSG D					
		14,224	61 >	75% Gras	s cover, Go	ood, HSG B				
		12,059	80 >	75% Gras	s cover, Go	ood, HSG D				
		1,928	98 V	Vater Surfa	ace, HSG D					
*		4,149	98 F	Parking Lot	, Walkways	s, Buildings etc.				
		74,951	74 V	Veighted A	verage					
		68,874	g	1.89% Per	vious Area					
		6,077	3	3.11% Impe	ervious Are	a				
	_	1 41.	01	V . I ! f	0	December 6 and				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	15.6	99	0.0404	0.11		Sheet Flow, A->B				
						Woods: Light underbrush n= 0.400 P2= 3.41"				
	2.7	161	0.0381	0.98		Shallow Concentrated Flow, B->C				
	Woodland Kv= 5.0 fps									
	0.2	48	0.0074	4.09	20.43	•				
				Area= 5.0 sf Perim= 7.0' r= 0.71'						
_						n= 0.025 Earth, clean & winding				
	18.5	308	Total							

Summary for Subcatchment 1A: Watershed 1A - Parking Lot

Runoff = 1.72 cfs @ 12.08 hrs, Volume= 0.121 af, Depth= 1.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Rainfall=2.78"

	Area (sf)	CN	Description
	6,211	61	>75% Grass cover, Good, HSG B
	2,124	80	>75% Grass cover, Good, HSG D
*	27,267	98	Parking Lot, Walkways, Buildings etc.
	35,602	90	Weighted Average
	8,335		23.41% Pervious Area
	27,267		76.59% Impervious Area

Prepared by Hudson Engineering & Consulting, P.C.

Printed 9/26/2018

Page 7

. Toparoa by Trades				
HydroCAD® 10.00-22	s/n 02549	© 2018 H	lydroCAD	Software Solutions LLC
•				

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	12	0.0075	0.05		Sheet Flow, A->B
						Grass: Dense n= 0.240 P2= 3.41"
	0.1	5	0.1140	1.56		Sheet Flow, B->C
						Smooth surfaces n= 0.011 P2= 3.41"
	1.1	82	0.0151	1.22		Sheet Flow, C->D
						Smooth surfaces n= 0.011 P2= 3.41"
	0.6	91	0.0176	2.69		Shallow Concentrated Flow, D->E
						Paved Kv= 20.3 fps
_	5.6	190	Total			

Summary for Subcatchment 1B: Watershed 1B - Roof Area

Runoff = 1.54 cfs @ 12.01 hrs, Volume= 0.102 af, Depth= 2.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Rainfall=2.78"

	Α	rea (sf)	CN	Description				
*		20,885	98	Proposed Building				
		20,885 100.00% Impervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	1.0					Direct Entry,		

Summary for Subcatchment 2: Watershed 2

Runoff = 0.23 cfs @ 12.25 hrs, Volume= 0.030 af, Depth= 0.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 1-Year Rainfall=2.78"

	Area (sf)	CN	Description
	13,317	58	Woods/grass comb., Good, HSG B
	6,548	67	Brush, Poor, HSG B
	10,764	61	>75% Grass cover, Good, HSG B
	368	98	Water Surface, HSG B
*	4,550	98	Parking Lot, Walkways, Buildings, etc.
	35,547	66	Weighted Average
	30,629		86.16% Pervious Area
	4,918		13.84% Impervious Area

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 8

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	13.2	77	0.0134	0.10		Sheet Flow, A->B Grass: Dense n= 0.240 P2= 3.41"	_
	0.1	32	0.5425	3.68		Shallow Concentrated Flow, B->C	
	0.8	64	0.0656	1.28		Woodland Kv= 5.0 fps Shallow Concentrated Flow, C->DP-2	
-	14.1	173	Total			Woodland Kv= 5.0 fps	-

Summary for Reach DP-1: DP-1

3.017 ac, 41.26% Impervious, Inflow Depth = 1.33" for 1-Year event Inflow Area =

Inflow 1.85 cfs @ 12.02 hrs, Volume= 0.334 af

Outflow 1.85 cfs @ 12.02 hrs, Volume= 0.334 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Summary for Reach DP-2: DP-2

0.816 ac, 13.84% Impervious, Inflow Depth = 0.44" for 1-Year event Inflow Area =

0.23 cfs @ 12.25 hrs, Volume= Inflow 0.030 af

Outflow 0.23 cfs @ 12.25 hrs, Volume= 0.030 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Summary for Pond 3P: Organic Filter (Type F-4) (100% WQv)

Inflow Area = 0.817 ac, 76.59% Impervious, Inflow Depth = 1.78" for 1-Year event

Inflow 1.72 cfs @ 12.08 hrs, Volume= 0.121 af

0.13 cfs @ 13.34 hrs, Volume= Outflow = 0.121 af, Atten= 92%, Lag= 75.6 min

0.13 cfs @ 13.34 hrs, Volume= 0.121 af Primary

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 301.62' @ 13.34 hrs Surf.Area= 2,147 sf Storage= 2,737 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 412.6 min (1,224.7 - 812.1)

Volume	Invert /	Avail.Storage	Storage [Description		
#1	300.00'	6,298 cf	Sedimen	tation Basin (Con	ic) Listed below (Re	calc)
Elevation (feet)	Surf.Ar (sq		c.Store c-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
300.00	1,2	:62	0	0	1,262	
301.00	1,7	'94	1,520	1,520	1,812	
302.00	2,3	81	2,081	3,601	2,420	
303.00	3,0	26	2,697	6,298	3,091	

Prepared by Hudson Engineering & Consulting, P.C.

Printed 9/26/2018

Page 9

HydroCAD® 10.00-22		Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	296.98'	12.0" Round 12" HDPE (OUT)
			L= 17.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.98' / 296.25' S= 0.0429 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	301.60'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	300.00'	1.375 in/hr Exfiltration over Surface area

Primary OutFlow Max=0.13 cfs @ 13.34 hrs HW=301.62' TW=296.48' (Dynamic Tailwater)

1=12" HDPE (OUT) (Passes 0.13 cfs of 6.07 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.06 cfs @ 0.44 fps)

—3=Exfiltration (Exfiltration Controls 0.07 cfs)

Summary for Pond 4P: Drainage Manhole

Inflow Area = 1.297 ac, 85.24% Impervious, Inflow Depth = 2.07" for 1-Year event

Inflow = 1.59 cfs @ 12.01 hrs, Volume= 0.223 af

Outflow = 1.59 cfs @ 12.01 hrs, Volume= 0.223 af, Atten= 0%, Lag= 0.0 min

Primary = 1.59 cfs @ 12.01 hrs, Volume= 0.223 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 296.90' @ 12.01 hrs

Flood Elev= 303.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	296.25'	18.0" Round Culvert
			L= 76.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.25' / 295.50' S= 0.0099 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.58 cfs @ 12.01 hrs HW=296.90' TW=0.00' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.58 cfs @ 2.16 fps)

Summary for Pond 5P: Hydrodynamic Separator WQv=0.75cfs, 100-year=5.21cfs

Inflow Area = 0.479 ac,100.00% Impervious, Inflow Depth = 2.55" for 1-Year event

Inflow = 1.54 cfs @ 12.01 hrs, Volume= 0.102 af

Outflow = 1.54 cfs @ 12.01 hrs, Volume= 0.102 af, Atten= 0%, Lag= 0.0 min

Primary = 1.54 cfs @ 12.01 hrs, Volume= 0.102 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 304.24' @ 12.01 hrs

Flood Elev= 307.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	303.47'	12.0" Round 12" HDPE (OUT)
			L= 123.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 303.47' / 296.25' S= 0.0587 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Type III 24-hr 1-Year Rainfall=2.78"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 10

Primary OutFlow Max=1.53 cfs @ 12.01 hrs HW=304.24' TW=296.90' (Dynamic Tailwater) 1=12" HDPE (OUT) (Inlet Controls 1.53 cfs @ 2.36 fps)

Summary for Pond 6P: Hydrodynamic Separator WQv=1.51cfs, 100-year=7.27cfs

Inflow Area = 0.817 ac, 76.59% Impervious, Inflow Depth = 1.78" for 1-Year event

Inflow = 1.72 cfs @ 12.08 hrs, Volume= 0.121 af

Outflow = 1.72 cfs @ 12.08 hrs, Volume= 0.121 af, Atten= 0%, Lag= 0.0 min

Primary = 1.72 cfs @ 12.08 hrs, Volume= 0.121 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 301.62' @ 13.33 hrs

Flood Elev= 303.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	300.70'	18.0" Round Culvert
	-		L= 35.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 300.70' / 300.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.72 cfs @ 12.08 hrs HW=301.38' TW=300.85' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.72 cfs @ 2.22 fps)

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 11

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Watershed 1 Runoff Area=74,951 sf 8.11% Impervious Runoff Depth=2.48" Flow Length=308' Tc=18.5 min CN=74 Runoff=3.46 cfs 0.355 af

Subcatchment 1A: Watershed 1A - Parking Runoff Area=35,602 sf 76.59% Impervious Runoff Depth=4.01" Flow Length=190' Tc=5.6 min CN=90 Runoff=3.75 cfs 0.273 af

Subcatchment 1B: Watershed 1B - Roof Runoff Area=20,885 sf 100.00% Impervious Runoff Depth=4.90" Tc=1.0 min CN=98 Runoff=2.87 cfs 0.196 af

Subcatchment 2: Watershed 2 Runoff Area=35,547 sf 13.84% Impervious Runoff Depth=1.82" Flow Length=173' Tc=14.1 min CN=66 Runoff=1.29 cfs 0.124 af

Reach DP-1: DP-1Inflow=7.01 cfs 0.824 af
Outflow=7.01 cfs 0.824 af

Reach DP-2: DP-2 Inflow=1.29 cfs 0.124 af

Outflow=1.29 cfs 0.124 af

Pond 3P: Organic Filter (Type F-4) (100% Peak Elev=301.85' Storage=3,260 cf Inflow=3.75 cfs 0.273 af

Outflow=3.43 cfs 0.273 af

Pond 4P: Drainage Manhole Peak Elev=297.53' Inflow=4.89 cfs 0.469 af

18.0" Round Culvert n=0.013 L=76.0' S=0.0099 '/' Outflow=4.89 cfs 0.469 af

Pond 5P: Hydrodynamic Separator WQv=0.75cfs, Peak Elev=304.90' Inflow=2.87 cfs 0.196 af

12.0" Round Culvert n=0.013 L=123.0' S=0.0587 '/' Outflow=2.87 cfs 0.196 af

Pond 6P: Hydrodynamic Separator WQv=1.51cfs, Peak Elev=302.16' Inflow=3.75 cfs 0.273 af

18.0" Round Culvert n=0.013 L=35.0' S=0.0200 '/' Outflow=3.75 cfs 0.273 af

Total Runoff Area = 3.833 ac Runoff Volume = 0.948 af Average Runoff Depth = 2.97" 64.58% Pervious = 2.476 ac 35.42% Impervious = 1.358 ac

Prepared by Hudson Engineering & Consulting, P.C.
HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 12

Summary for Subcatchment 1: Watershed 1

Runoff = 3.46 cfs @ 12.27 hrs, Volume= 0.355 af, Depth= 2.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.14"

	Α	rea (sf)	CN Description					
		7,667	55 V	Voods, Go	od, HSG B			
		34,924	77 V	Voods, Go	od, HSG D			
		14,224	61 >	61 >75% Grass cover, Good, HSG B				
		12,059	80 >	80 >75% Grass cover, Good, HSG D				
		1,928 98 Water Surface, HSG D						
*		4,149	98 F	Parking Lot	, Walkways	s, Buildings etc.		
		74,951	74 V	Veighted A	verage			
		68,874	9	1.89% Per	vious Area			
		6,077	8	3.11% Impe	ervious Area	a		
	_							
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	15.6	99	0.0404	0.11		Sheet Flow, A->B		
						Woods: Light underbrush n= 0.400 P2= 3.41"		
	2.7	161	0.0381	0.98		Shallow Concentrated Flow, B->C		
						Woodland Kv= 5.0 fps		
	0.2	48	0.0074	4.09	20.43	Channel Flow, C->DP-1		
						Area= 5.0 sf Perim= 7.0' r= 0.71'		
_						n= 0.025 Earth, clean & winding		
	18.5	308	Total					

Summary for Subcatchment 1A: Watershed 1A - Parking Lot

Runoff = 3.75 cfs @ 12.08 hrs, Volume= 0.273 af, Depth= 4.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.14"

	Area (sf)	CN	Description
	6,211	61	>75% Grass cover, Good, HSG B
	2,124	80	>75% Grass cover, Good, HSG D
*	27,267	98	Parking Lot, Walkways, Buildings etc.
	35,602	90	Weighted Average
	8,335		23.41% Pervious Area
	27,267		76.59% Impervious Area

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 13

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.8	12	0.0075	0.05		Sheet Flow, A->B
					Grass: Dense n= 0.240 P2= 3.41"
0.1	5	0.1140	1.56		Sheet Flow, B->C
					Smooth surfaces n= 0.011 P2= 3.41"
1.1	82	0.0151	1.22		Sheet Flow, C->D
					Smooth surfaces n= 0.011 P2= 3.41"
0.6	91	0.0176	2.69		Shallow Concentrated Flow, D->E
					Paved Kv= 20.3 fps
 5.6	190	Total			

Summary for Subcatchment 1B: Watershed 1B - Roof Area

Runoff = 2.87 cfs @ 12.01 hrs, Volume=

0.196 af, Depth= 4.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.14"

	Α	rea (sf)	CN I	CN Description				
*		20,885	98 I	98 Proposed Building				
		20,885	•	100.00% Im	npervious A	Area		
	Тс	Length	Slope	,		Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	1.0					Direct Entry,		

Summary for Subcatchment 2: Watershed 2

Runoff = 1.29 cfs @ 12.20 hrs, Volume=

0.124 af, Depth= 1.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.14"

	Area (sf)	CN	Description		
	13,317	58	Woods/grass comb., Good, HSG B		
	6,548 67 Brush, Poor, HSG B				
	10,764	4 61 >75% Grass cover, Good, HSG B			
	368	98	Water Surface, HSG B		
*	4,550	98	Parking Lot, Walkways, Buildings, etc.		
	35,547	66	Weighted Average		
	30,629		86.16% Pervious Area		
	4,918		13.84% Impervious Area		

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC Printed 9/26/2018

Page 14

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
13.2	77	0.0134	0.10		Sheet Flow, A->B
					Grass: Dense n= 0.240 P2= 3.41"
0.1	32	0.5425	3.68		Shallow Concentrated Flow, B->C
					Woodland Kv= 5.0 fps
8.0	64	0.0656	1.28		Shallow Concentrated Flow, C->DP-2
					Woodland Kv= 5.0 fps
14.1	173	Total			

Summary for Reach DP-1: DP-1

3.017 ac, 41.26% Impervious, Inflow Depth = 3.28" for 10-Year event Inflow Area =

Inflow 7.01 cfs @ 12.14 hrs, Volume= 0.824 af

Outflow 7.01 cfs @ 12.14 hrs, Volume= 0.824 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Summary for Reach DP-2: DP-2

0.816 ac, 13.84% Impervious, Inflow Depth = 1.82" for 10-Year event Inflow Area =

1.29 cfs @ 12.20 hrs, Volume= Inflow 0.124 af

Outflow 1.29 cfs @ 12.20 hrs, Volume= 0.124 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Summary for Pond 3P: Organic Filter (Type F-4) (100% WQv)

Inflow Area = 0.817 ac, 76.59% Impervious, Inflow Depth = 4.01" for 10-Year event

Inflow 3.75 cfs @ 12.08 hrs, Volume= 0.273 af

3.43 cfs @ 12.11 hrs, Volume= Outflow = 0.273 af, Atten= 9%, Lag= 2.1 min

3.43 cfs @ 12.11 hrs, Volume= 0.273 af Primary

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 301.85' @ 12.11 hrs Surf.Area= 2,290 sf Storage= 3,260 cf

Plug-Flow detention time= 239.5 min calculated for 0.273 af (100% of inflow)

Center-of-Mass det. time= 239.6 min (1,029.0 - 789.4)

Volume	Invert A	Avail.Storage	Storage I	Description		
#1	300.00'	6,298 cf	Sedimen	ntation Basin (Con	ic) Listed below (Rec	alc)
Elevation (feet)	Surf.Ar (sq-		c.Store c-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
300.00	1,2	62	0	0	1,262	
301.00	1,7	94	1,520	1,520	1,812	
302.00	2,3	81	2,081	3,601	2,420	
303.00	3,0	26	2,697	6,298	3,091	

Device 1

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 15

Device	Routing	Invert	Outlet Devices
#1	Primary	296.98'	12.0" Round 12" HDPE (OUT)
	•		L= 17.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.98' / 296.25' S= 0.0429 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	301.60'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

300.00' 1.375 in/hr Exfiltration over Surface area

Primary OutFlow Max=3.42 cfs @ 12.11 hrs HW=301.85' TW=297.48' (Dynamic Tailwater)

1=12" HDPE (OUT) (Passes 3.42 cfs of 6.24 cfs potential flow)

2=Orifice/Grate (Weir Controls 3.35 cfs @ 1.65 fps)

—3=Exfiltration (Exfiltration Controls 0.07 cfs)

Summary for Pond 4P: Drainage Manhole

Inflow Area = 1.297 ac, 85.24% Impervious, Inflow Depth = 4.34" for 10-Year event

Inflow = 4.89 cfs @ 12.08 hrs, Volume= 0.469 af

Outflow = 4.89 cfs @ 12.08 hrs, Volume= 0.469 af, Atten= 0%, Lag= 0.0 min

Primary = 4.89 cfs @ 12.08 hrs, Volume= 0.469 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 297.53' @ 12.08 hrs

Flood Elev= 303.00'

#3

Device	Routing	Invert	Outlet Devices
#1	Primary	296.25'	18.0" Round Culvert
			L= 76.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.25' / 295.50' S= 0.0099 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.88 cfs @ 12.08 hrs HW=297.53' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.88 cfs @ 3.04 fps)

Summary for Pond 5P: Hydrodynamic Separator WQv=0.75cfs, 100-year=5.21cfs

Inflow Area = 0.479 ac,100.00% Impervious, Inflow Depth = 4.90" for 10-Year event

Inflow = 2.87 cfs @ 12.01 hrs, Volume= 0.196 af

Outflow = 2.87 cfs @ 12.01 hrs, Volume= 0.196 af, Atten= 0%, Lag= 0.0 min

Primary = 2.87 cfs @ 12.01 hrs, Volume= 0.196 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 304.90' @ 12.01 hrs

Flood Elev= 307.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	303.47'	12.0" Round 12" HDPE (OUT)
			L= 123.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 303.47' / 296.25' S= 0.0587 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Type III 24-hr 10-Year Rainfall=5.14"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 16

Primary OutFlow Max=2.85 cfs @ 12.01 hrs HW=304.88' TW=297.32' (Dynamic Tailwater) 1=12" HDPE (OUT) (Inlet Controls 2.85 cfs @ 3.63 fps)

Summary for Pond 6P: Hydrodynamic Separator WQv=1.51cfs, 100-year=7.27cfs

Inflow Area = 0.817 ac, 76.59% Impervious, Inflow Depth = 4.01" for 10-Year event

Inflow = 3.75 cfs @ 12.08 hrs, Volume= 0.273 af

Outflow = 3.75 cfs @ 12.08 hrs, Volume= 0.273 af, Atten= 0%, Lag= 0.0 min

Primary = 3.75 cfs @ 12.08 hrs, Volume= 0.273 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 302.16' @ 12.09 hrs

Flood Elev= 303.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	300.70'	18.0" Round Culvert
			L= 35.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 300.70' / 300.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=3.75 cfs @ 12.08 hrs HW=302.15' TW=301.84' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.75 cfs @ 2.14 fps)

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 17

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Watershed 1 Runoff Area=74,951 sf 8.11% Impervious Runoff Depth=3.60"

Flow Length=308' Tc=18.5 min CN=74 Runoff=5.05 cfs 0.516 af

Subcatchment 1A: Watershed 1A - Parking Runoff Area=35,602 sf 76.59% Impervious Runoff Depth=5.32"

Flow Length=190' Tc=5.6 min CN=90 Runoff=4.91 cfs 0.363 af

Subcatchment 1B: Watershed 1B - Roof Runoff Area=20,885 sf 100.00% Impervious Runoff Depth=6.25" Tc=1.0 min CN=98 Runoff=3.64 cfs 0.250 af

Subcatchment 2: Watershed 2 Runoff Area=35,547 sf 13.84% Impervious Runoff Depth=2.81"

Flow Length=173' Tc=14.1 min CN=66 Runoff=2.05 cfs 0.191 af

Reach DP-1: DP-1 Inflow=9.64 cfs 1.129 af

Outflow=9.64 cfs 1.129 af

Reach DP-2: DP-2 Inflow=2.05 cfs 0.191 af

Outflow=2.05 cfs 0.191 af

Pond 3P: Organic Filter (Type F-4) (100% Peak Elev=301.91' Storage=3,393 cf Inflow=4.91 cfs 0.363 af

Outflow=4.63 cfs 0.363 af

Pond 4P: Drainage Manhole Peak Elev=298.00' Inflow=6.73 cfs 0.612 af

18.0" Round Culvert n=0.013 L=76.0' S=0.0099 '/' Outflow=6.73 cfs 0.612 af

Pond 5P: Hydrodynamic Separator WQv=0.75cfs, Peak Elev=305.45' Inflow=3.64 cfs 0.250 af

12.0" Round Culvert n=0.013 L=123.0' S=0.0587 '/' Outflow=3.64 cfs 0.250 af

Pond 6P: Hydrodynamic Separator WQv=1.51cfs, Peak Elev=302.44' Inflow=4.91 cfs 0.363 af

18.0" Round Culvert n=0.013 L=35.0' S=0.0200 '/' Outflow=4.91 cfs 0.363 af

Total Runoff Area = 3.833 ac Runoff Volume = 1.320 af Average Runoff Depth = 4.13" 64.58% Pervious = 2.476 ac 35.42% Impervious = 1.358 ac Prepared by Hudson Engineering & Consulting, P.C.
HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 18

Summary for Subcatchment 1: Watershed 1

Runoff = 5.05 cfs @ 12.25 hrs, Volume= 0.516 af, Depth= 3.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.49"

	Α	rea (sf)	CN E	escription						
		7,667	55 V	Voods, Go	od, HSG B					
		34,924	77 V	Voods, Go	od, HSG D					
		14,224	61 >	>75% Grass cover, Good, HSG B						
		12,059	80 >	>75% Grass cover, Good, HSG D						
		1,928	98 V	Water Surface, HSG D						
*		4,149	98 F	Parking Lot	, Walkways	s, Buildings etc.				
		74,951	74 V	Veighted A	verage					
		68,874	9	1.89% Per	vious Area					
		6,077	8	3.11% Impe	ervious Area	a				
	_									
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	15.6	99	0.0404	0.11		Sheet Flow, A->B				
						Woods: Light underbrush n= 0.400 P2= 3.41"				
	2.7	161	0.0381	0.98		Shallow Concentrated Flow, B->C				
						Woodland Kv= 5.0 fps				
	0.2	48	0.0074	4.09	20.43	Channel Flow, C->DP-1				
						Area= 5.0 sf Perim= 7.0' r= 0.71'				
_						n= 0.025 Earth, clean & winding				
	18.5	308	Total							

Summary for Subcatchment 1A: Watershed 1A - Parking Lot

Runoff = 4.91 cfs @ 12.08 hrs, Volume= 0.363 af, Depth= 5.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.49"

	Area (sf)	CN	Description
	6,211	61	>75% Grass cover, Good, HSG B
	2,124	80	>75% Grass cover, Good, HSG D
*	27,267	98	Parking Lot, Walkways, Buildings etc.
	35,602	90	Weighted Average
	8,335		23.41% Pervious Area
	27,267		76.59% Impervious Area

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 19

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	12	0.0075	0.05		Sheet Flow, A->B
						Grass: Dense n= 0.240 P2= 3.41"
	0.1	5	0.1140	1.56		Sheet Flow, B->C
						Smooth surfaces n= 0.011 P2= 3.41"
	1.1	82	0.0151	1.22		Sheet Flow, C->D
						Smooth surfaces n= 0.011 P2= 3.41"
	0.6	91	0.0176	2.69		Shallow Concentrated Flow, D->E
						Paved Kv= 20.3 fps
_	5.6	190	Total			

Summary for Subcatchment 1B: Watershed 1B - Roof Area

Runoff = 3.64 cfs @ 12.01 hrs, Volume= 0.250 af, Depth= 6.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.49"

	Α	rea (sf)	CN I	Description			
*		20,885	98 I	8 Proposed Building			
		20,885	•	100.00% Impervious Area			
	Тс	Length	Slope	,		Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	1.0					Direct Entry,	

Summary for Subcatchment 2: Watershed 2

Runoff = 2.05 cfs @ 12.20 hrs, Volume= 0.191 af, Depth= 2.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.49"

	Area (sf)	CN	Description
	13,317	58	Woods/grass comb., Good, HSG B
	6,548	67	Brush, Poor, HSG B
	10,764	61	>75% Grass cover, Good, HSG B
	368	98	Water Surface, HSG B
*	4,550	98	Parking Lot, Walkways, Buildings, etc.
	35,547	66	Weighted Average
	30,629		86.16% Pervious Area
	4,918		13.84% Impervious Area

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC Printed 9/26/2018

Page 20

	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
	13.2	77	0.0134	0.10		Sheet Flow, A->B
						Grass: Dense n= 0.240 P2= 3.41"
	0.1	32	0.5425	3.68		Shallow Concentrated Flow, B->C
						Woodland Kv= 5.0 fps
	8.0	64	0.0656	1.28		Shallow Concentrated Flow, C->DP-2
_						Woodland Kv= 5.0 fps
	14.1	173	Total			

Summary for Reach DP-1: DP-1

3.017 ac, 41.26% Impervious, Inflow Depth = 4.49" for 25-Year event Inflow Area =

Inflow 9.64 cfs @ 12.14 hrs, Volume= 1.129 af

Outflow 9.64 cfs @ 12.14 hrs, Volume= 1.129 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Summary for Reach DP-2: DP-2

0.816 ac, 13.84% Impervious, Inflow Depth = 2.81" for 25-Year event Inflow Area =

2.05 cfs @ 12.20 hrs, Volume= Inflow 0.191 af

Outflow 2.05 cfs @ 12.20 hrs, Volume= 0.191 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Summary for Pond 3P: Organic Filter (Type F-4) (100% WQv)

Inflow Area = 0.817 ac, 76.59% Impervious, Inflow Depth = 5.32" for 25-Year event

Inflow 4.91 cfs @ 12.08 hrs, Volume= 0.363 af

4.63 cfs @ 12.11 hrs, Volume= Outflow = 0.363 af, Atten= 6%, Lag= 1.7 min

4.63 cfs @ 12.11 hrs, Volume= 0.363 af Primary

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 301.91' @ 12.11 hrs Surf.Area= 2,326 sf Storage= 3,393 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 198.0 min (979.9 - 781.8)

Volume	Invert A	Avail.Storage	Storage I	Description		
#1	300.00'	6,298 cf	Sedimen	ntation Basin (Con	ic) Listed below (Rec	alc)
Elevation (feet)	Surf.Ar (sq-		c.Store c-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
300.00	1,2	62	0	0	1,262	
301.00	1,7	94	1,520	1,520	1,812	
302.00	2,3	81	2,081	3,601	2,420	
303.00	3,0	26	2,697	6,298	3,091	

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 21

Device	Routing	Invert	Outlet Devices
#1	Primary	296.98'	12.0" Round 12" HDPE (OUT)
			L= 17.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.98' / 296.25' S= 0.0429 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	301.60'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	300 00'	1.375 in/hr Exfiltration over Surface area

Primary OutFlow Max=4.62 cfs @ 12.11 hrs HW=301.91' TW=297.88' (Dynamic Tailwater)

_1=12" HDPE (OUT) (Passes 4.62 cfs of 5.99 cfs potential flow)

1—2=Orifice/Grate (Weir Controls 4.55 cfs @ 1.83 fps)

—3=Exfiltration (Exfiltration Controls 0.07 cfs)

Summary for Pond 4P: Drainage Manhole

Inflow Area = 1.297 ac, 85.24% Impervious, Inflow Depth = 5.67" for 25-Year event

Inflow = 6.73 cfs @ 12.07 hrs, Volume= 0.612 af

Outflow = 6.73 cfs @ 12.07 hrs, Volume= 0.612 af, Atten= 0%, Lag= 0.0 min

Primary = 6.73 cfs @ 12.07 hrs, Volume= 0.612 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 298.00' @ 12.07 hrs

Flood Elev= 303.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	296.25'	18.0" Round Culvert
			L= 76.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.25' / 295.50' S= 0.0099 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=6.72 cfs @ 12.07 hrs HW=298.00' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 6.72 cfs @ 3.81 fps)

Summary for Pond 5P: Hydrodynamic Separator WQv=0.75cfs, 100-year=5.21cfs

Inflow Area = 0.479 ac,100.00% Impervious, Inflow Depth = 6.25" for 25-Year event

Inflow = 3.64 cfs @ 12.01 hrs, Volume= 0.250 af

Outflow = 3.64 cfs @ 12.01 hrs, Volume= 0.250 af, Atten= 0%, Lag= 0.0 min

Primary = 3.64 cfs @ 12.01 hrs, Volume= 0.250 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 305.45' @ 12.01 hrs

Flood Elev= 307.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	303.47'	12.0" Round 12" HDPE (OUT)
			L= 123.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 303.47' / 296.25' S= 0.0587 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Type III 24-hr 25-Year Rainfall=6.49"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 22

Primary OutFlow Max=3.61 cfs @ 12.01 hrs HW=305.43' TW=297.90' (Dynamic Tailwater) 1=12" HDPE (OUT) (Inlet Controls 3.61 cfs @ 4.60 fps)

Summary for Pond 6P: Hydrodynamic Separator WQv=1.51cfs, 100-year=7.27cfs

Inflow Area = 0.817 ac, 76.59% Impervious, Inflow Depth = 5.32" for 25-Year event

Inflow = 4.91 cfs @ 12.08 hrs, Volume= 0.363 af

Outflow = 4.91 cfs @ 12.08 hrs, Volume= 0.363 af, Atten= 0%, Lag= 0.0 min

Primary = 4.91 cfs @ 12.08 hrs, Volume= 0.363 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 302.44' @ 12.08 hrs

Flood Elev= 303.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	300.70'	18.0" Round Culvert
	-		L= 35.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 300.70' / 300.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.90 cfs @ 12.08 hrs HW=302.43' TW=301.90' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.90 cfs @ 2.77 fps)

Type III 24-hr 100-Year Rainfall=9.29"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC Printed 9/26/2018

Page 23

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points x 3 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Watershed 1

Runoff Area=74,951 sf 8.11% Impervious Runoff Depth=6.09"

Flow Length=308' Tc=18.5 min CN=74 Runoff=8.52 cfs 0.874 af

Subcatchment 1A: Watershed 1A - Parking Runoff Area=35,602 sf 76.59% Impervious Runoff Depth=8.08" Flow Length=190' Tc=5.6 min CN=90 Runoff=7.27 cfs 0.550 af

Subcatchment 1B: Watershed 1B - Roof Runoff Area=20,885 sf 100.00% Impervious Runoff Depth=9.05"

Tc=1.0 min CN=98 Runoff=5.21 cfs 0.362 af

Subcatchment 2: Watershed 2

Runoff Area=35.547 sf 13.84% Impervious Runoff Depth=5.09" Flow Length=173' Tc=14.1 min CN=66 Runoff=3.77 cfs 0.346 af

Reach DP-1: DP-1

Inflow=15.91 cfs 1.786 af

Outflow=15.91 cfs 1.786 af

Reach DP-2: DP-2

Inflow=3.77 cfs 0.346 af Outflow=3.77 cfs 0.346 af

Pond 3P: Organic Filter (Type F-4) (100%

Peak Elev=302.13' Storage=3,916 cf Inflow=7.27 cfs 0.550 af

Outflow=5.81 cfs 0.550 af

Pond 4P: Drainage Manhole

Peak Elev=299.10' Inflow=9.74 cfs 0.912 af

18.0" Round Culvert n=0.013 L=76.0' S=0.0099 '/' Outflow=9.74 cfs 0.912 af

Pond 5P: Hydrodynamic Separator WQv=0.75cfs,

Peak Elev=307.02' Inflow=5.21 cfs 0.362 af

12.0" Round Culvert n=0.013 L=123.0' S=0.0587 '/' Outflow=5.21 cfs 0.362 af

Pond 6P: Hydrodynamic Separator WQv=1.51cfs,

Peak Elev=303.22' Inflow=7.27 cfs 0.550 af

18.0" Round Culvert n=0.013 L=35.0' S=0.0200 '/' Outflow=7.27 cfs 0.550 af

Total Runoff Area = 3.833 ac Runoff Volume = 2.131 af Average Runoff Depth = 6.67" 64.58% Pervious = 2.476 ac 35.42% Impervious = 1.358 ac

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 24

Summary for Subcatchment 1: Watershed 1

Runoff = 8.52 cfs @ 12.25 hrs, Volume= 0.874 af, Depth= 6.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.29"

	Α	rea (sf)	CN E	escription		
		7,667	55 V	Voods, Go	od, HSG B	
		34,924	77 V	Voods, Go	od, HSG D	
		14,224	61 >	75% Gras	s cover, Go	ood, HSG B
		12,059	80 >	75% Gras	s cover, Go	ood, HSG D
		1,928	98 V	Vater Surfa	ace, HSG D	
*		4,149	98 F	Parking Lot	, Walkways	s, Buildings etc.
		74,951	74 V	Veighted A	verage	
		68,874	9	1.89% Per	vious Area	
		6,077	8	3.11% Impe	ervious Are	а
	_					
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.6	99	0.0404	0.11		Sheet Flow, A->B
						Woods: Light underbrush n= 0.400 P2= 3.41"
	2.7	161	0.0381	0.98		Shallow Concentrated Flow, B->C
						Woodland Kv= 5.0 fps
	0.2	48	0.0074	4.09	20.43	Channel Flow, C->DP-1
						Area= 5.0 sf Perim= 7.0' r= 0.71'
_						n= 0.025 Earth, clean & winding
	18.5	308	Total			

Summary for Subcatchment 1A: Watershed 1A - Parking Lot

Runoff = 7.27 cfs @ 12.08 hrs, Volume= 0.550 af, Depth= 8.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.29"

	Area (sf)	CN	Description
	6,211	61	>75% Grass cover, Good, HSG B
	2,124	80	>75% Grass cover, Good, HSG D
*	27,267	98	Parking Lot, Walkways, Buildings etc.
	35,602	90	Weighted Average
	8,335		23.41% Pervious Area
	27,267		76.59% Impervious Area

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Pag	е	25
_		

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.8	12	0.0075	0.05		Sheet Flow, A->B
						Grass: Dense n= 0.240 P2= 3.41"
	0.1	5	0.1140	1.56		Sheet Flow, B->C
						Smooth surfaces n= 0.011 P2= 3.41"
	1.1	82	0.0151	1.22		Sheet Flow, C->D
						Smooth surfaces n= 0.011 P2= 3.41"
	0.6	91	0.0176	2.69		Shallow Concentrated Flow, D->E
						Paved Kv= 20.3 fps
_	5.6	190	Total			

Summary for Subcatchment 1B: Watershed 1B - Roof Area

Runoff = 5.21 cfs @ 12.01 hrs, Volume= 0.362 af, Depth= 9.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.29"

	Α	rea (sf)	CN	Description		
*		20,885	98	Proposed E	Building	
		20,885		100.00% In	npervious A	Area
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	1.0					Direct Entry,

Summary for Subcatchment 2: Watershed 2

Runoff = 3.77 cfs @ 12.20 hrs, Volume= 0.346 af, Depth= 5.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.29"

	Area (sf)	CN	Description
	13,317	58	Woods/grass comb., Good, HSG B
	6,548	67	Brush, Poor, HSG B
	10,764	61	>75% Grass cover, Good, HSG B
	368	98	Water Surface, HSG B
*	4,550	98	Parking Lot, Walkways, Buildings, etc.
	35,547	66	Weighted Average
	30,629		86.16% Pervious Area
	4,918		13.84% Impervious Area

Prepared by Hudson Engineering & Consulting, P.C.

Printed 9/26/2018

HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Page 26

	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	13.2	77	0.0134	0.10		Sheet Flow, A->B
						Grass: Dense n= 0.240 P2= 3.41"
	0.1	32	0.5425	3.68		Shallow Concentrated Flow, B->C
						Woodland Kv= 5.0 fps
	8.0	64	0.0656	1.28		Shallow Concentrated Flow, C->DP-2
_						Woodland Kv= 5.0 fps
	14.1	173	Total			

Summary for Reach DP-1: DP-1

3.017 ac, 41.26% Impervious, Inflow Depth = 7.10" for 100-Year event Inflow Area =

Inflow 15.91 cfs @ 12.22 hrs, Volume= 1.786 af

Outflow 15.91 cfs @ 12.22 hrs, Volume= 1.786 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Summary for Reach DP-2: DP-2

0.816 ac, 13.84% Impervious, Inflow Depth = 5.09" for 100-Year event Inflow Area =

3.77 cfs @ 12.20 hrs, Volume= Inflow 0.346 af

Outflow 3.77 cfs @ 12.20 hrs, Volume= 0.346 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Summary for Pond 3P: Organic Filter (Type F-4) (100% WQv)

0.817 ac, 76.59% Impervious, Inflow Depth = 8.08" for 100-Year event Inflow Area =

7.27 cfs @ 12.08 hrs, Volume= Inflow 0.550 af

5.81 cfs @ 12.16 hrs, Volume= Outflow = 0.550 af, Atten= 20%, Lag= 4.9 min

5.81 cfs @ 12.16 hrs, Volume= 0.550 af Primary

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 302.13' @ 12.14 hrs Surf.Area= 2,461 sf Storage= 3,916 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 145.1 min (916.4 - 771.3)

Volume	Invert A	Avail.Storage	Storage I	Description		
#1	300.00'	6,298 cf	Sedimen	ntation Basin (Con	ic) Listed below (Rec	alc)
Elevation (feet)	Surf.Ar (sq-		c.Store c-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
300.00	1,2	62	0	0	1,262	
301.00	1,7	94	1,520	1,520	1,812	
302.00	2,3	81	2,081	3,601	2,420	
303.00	3,0	26	2,697	6,298	3,091	

Prepared by Hudson Engineering & Consulting, P.C.

Printed 9/26/2018

Page 27

HydroCAD®	10.00-22	s/n 02549	© 201	8 Hydro	CAD Software Sol	utions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	296.98'	12.0" Round 12" HDPE (OUT)
	•		L= 17.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.98' / 296.25' S= 0.0429 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	301.60'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	300.00'	1.375 in/hr Exfiltration over Surface area

Primary OutFlow Max=5.81 cfs @ 12.16 hrs HW=302.11' TW=298.33' (Dynamic Tailwater)

1=12" HDPE (OUT) (Inlet Controls 5.81 cfs @ 7.40 fps)

2=Orifice/Grate (Passes < 9.62 cfs potential flow)

—3=Exfiltration (Passes < 0.08 cfs potential flow)

Summary for Pond 4P: Drainage Manhole

Inflow Area = 1.297 ac, 85.24% Impervious, Inflow Depth = 8.44" for 100-Year event

Inflow = 9.74 cfs @ 12.03 hrs, Volume= 0.912 af

Outflow = 9.74 cfs @ 12.03 hrs, Volume= 0.912 af, Atten= 0%, Lag= 0.0 min

Primary = 9.74 cfs @ 12.03 hrs, Volume= 0.912 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 299.10' @ 12.03 hrs

Flood Elev= 303.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	296.25'	18.0" Round Culvert
			L= 76.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.25' / 295.50' S= 0.0099 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=9.73 cfs @ 12.03 hrs HW=299.10' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 9.73 cfs @ 5.51 fps)

Summary for Pond 5P: Hydrodynamic Separator WQv=0.75cfs, 100-year=5.21cfs

Inflow Area = 0.479 ac,100.00% Impervious, Inflow Depth = 9.05" for 100-Year event

Inflow = 5.21 cfs @ 12.01 hrs, Volume= 0.362 af

Outflow = 5.21 cfs @ 12.01 hrs, Volume= 0.362 af, Atten= 0%, Lag= 0.0 min

Primary = 5.21 cfs @ 12.01 hrs, Volume= 0.362 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 307.02' @ 12.01 hrs

Flood Elev= 307.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	303.47'	12.0" Round 12" HDPE (OUT)
			L= 123.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 303.47' / 296.25' S= 0.0587 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Type III 24-hr 100-Year Rainfall=9.29"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 28

Primary OutFlow Max=5.18 cfs @ 12.01 hrs HW=306.98' TW=298.97' (Dynamic Tailwater) 1=12" HDPE (OUT) (Inlet Controls 5.18 cfs @ 6.59 fps)

Summary for Pond 6P: Hydrodynamic Separator WQv=1.51cfs, 100-year=7.27cfs

Inflow Area = 0.817 ac, 76.59% Impervious, Inflow Depth = 8.08" for 100-Year event

Inflow = 7.27 cfs @ 12.08 hrs, Volume= 0.550 af

Outflow = 7.27 cfs @ 12.08 hrs, Volume= 0.550 af, Atten= 0%, Lag= 0.0 min

Primary = 7.27 cfs @ 12.08 hrs, Volume= 0.550 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

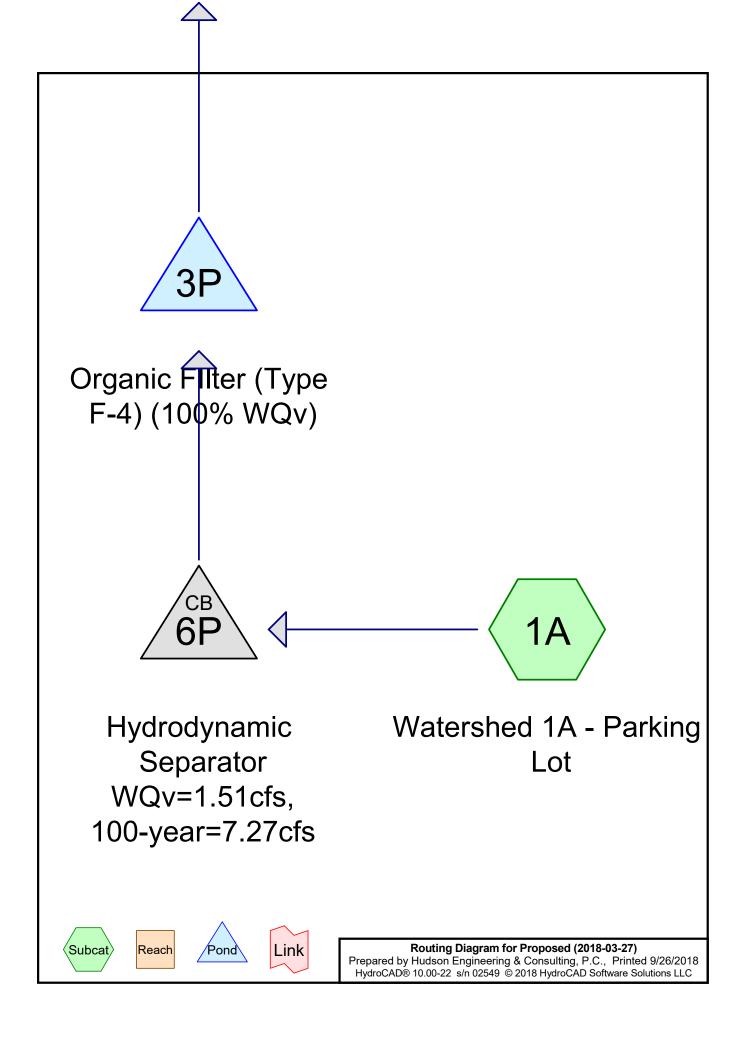
Peak Elev= 303.22' @ 12.09 hrs

Flood Elev= 303.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	300.70'	18.0" Round Culvert
			L= 35.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 300.70' / 300.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=7.26 cfs @ 12.08 hrs HW=303.21' TW=302.04' (Dynamic Tailwater) —1=Culvert (Inlet Controls 7.26 cfs @ 4.11 fps)

9.) Water Quality Calculations	
Hudson Engineering & Consulting, P.C.	



Printed 9/26/2018 Page 2

Area Listing (selected nodes)

Area	CN	Description
 (acres)		(subcatchment-numbers)
0.143	61	>75% Grass cover, Good, HSG B (1A)
0.049	80	>75% Grass cover, Good, HSG D (1A)
0.626	98	Parking Lot, Walkways, Buildings etc. (1A)
0.817	90	TOTAL AREA

Printed 9/26/2018 Page 3

Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.143	HSG B	1A
0.000	HSG C	
0.049	HSG D	1A
0.626	Other	1A
0.817		TOTAL AREA

Printed 9/26/2018 Page 4

> Subcatch **Numbers**

Ground Covers (selected nodes)

HSC	S-A HS	G-B H	SG-C H	SG-D	Other	Total	Ground
(acr	es) (ad	res) (a	icres) (a	icres) (acres) ((acres)	Cover
0.0	000 0	.143	0.000	0.049	0.000	0.191	>75% Grass cover, Good
0.0	000 0	.000	0.000	0.000	0.626	0.626	Parking Lot, Walkways, Buildings
							etc.
0.0	000 0	.143	0.000	0.049	0.626	0.817	TOTAL AREA

Type III 24-hr 100-Year Rainfall=9.29"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 5

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1A: Watershed 1A - Parking Runoff Area=35,602 sf 76.59% Impervious Runoff Depth=8.08" Flow Length=190' Tc=5.6 min CN=90 Runoff=7.27 cfs 0.550 af

Pond 3P: Organic Filter (Type F-4) (100% Peak Elev=302.13' Storage=3,916 cf Inflow=7.27 cfs 0.550 af Outflow=5.81 cfs 0.550 af

Pond 6P: Hydrodynamic Separator WQv=1.51cfs,Peak Elev=303.22' Inflow=7.27 cfs 0.550 af 18.0" Round Culvert n=0.013 L=35.0' S=0.0200 '/' Outflow=7.27 cfs 0.550 af

Total Runoff Area = 0.817 ac Runoff Volume = 0.550 af Average Runoff Depth = 8.08" 23.41% Pervious = 0.191 ac 76.59% Impervious = 0.626 ac

Printed 9/26/2018

Page 6

Summary for Subcatchment 1A: Watershed 1A - Parking Lot

Runoff 7.27 cfs @ 12.08 hrs, Volume= 0.550 af, Depth= 8.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.29"

	Α	rea (sf)	CN I	Description					
		6,211 61 >75% Grass cover, Good, HSG B							
		2,124	80 :	>75% Gras	s cover, Go	ood, HSG D			
*		27,267	98 I	Parking Lot	, Walkways	s, Buildings etc.			
		35,602	90 \	Neighted A	verage				
		8,335		23.41% Per	vious Area				
		27,267	-	76.59% Imp	ervious Ar	ea			
				_					
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.8	12	0.0075	0.05		Sheet Flow, A->B			
						Grass: Dense n= 0.240 P2= 3.41"			
	0.1	5	0.1140	1.56		Sheet Flow, B->C			
						Smooth surfaces n= 0.011 P2= 3.41"			
	1.1	82	0.0151	1.22		Sheet Flow, C->D			
						Smooth surfaces n= 0.011 P2= 3.41"			
	0.6	91	0.0176	2.69		Shallow Concentrated Flow, D->E			
_						Paved Kv= 20.3 fps			
	5.6	190	Total						

Summary for Pond 3P: Organic Filter (Type F-4) (100% WQv)

0.817 ac, 76.59% Impervious, Inflow Depth = 8.08" for 100-Year event Inflow Area = Inflow 7.27 cfs @ 12.08 hrs, Volume= 0.550 af

Outflow 5.81 cfs @ 12.16 hrs, Volume= 0.550 af, Atten= 20%, Lag= 4.9 min

5.81 cfs @ 12.16 hrs, Volume= Primary 0.550 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 302.13' @ 12.14 hrs Surf.Area= 2,461 sf Storage= 3,916 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 145.1 min (916.4 - 771.3)

Volume	Invert	Avail.Storag	e Storage	Description		
#1	300.00'	6,298	of Sedime	ntation Basin (Con	ic) Listed below (Recalc)
Elevation (feet)	Surf. <i>l</i> (s		Inc.Store ubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
300.00		,262	0	0	1,262	
301.00		,794	1,520	1,520	1,812	
302.00 303.00		,381 ,026	2,081 2,697	3,601 6,298	2,420 3,091	

Prepared by Hudson Engineering & Consulting, P.C.

Printed 9/26/2018

HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Page 7

Device	Routing	Invert	Outlet Devices
#1	Primary	296.98'	12.0" Round 12" HDPE (OUT)
	•		L= 17.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.98' / 296.25' S= 0.0429 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	301.60'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	300.00'	1.375 in/hr Exfiltration over Surface area

Primary OutFlow Max=5.81 cfs @ 12.16 hrs HW=302.11' TW=298.33' (Dynamic Tailwater)

1=12" HDPE (OUT) (Inlet Controls 5.81 cfs @ 7.40 fps)

2=Orifice/Grate (Passes < 9.62 cfs potential flow)

3=Exfiltration (Passes < 0.08 cfs potential flow)

Summary for Pond 6P: Hydrodynamic Separator WQv=1.51cfs, 100-year=7.27cfs

Inflow Area = 0.817 ac, 76.59% Impervious, Inflow Depth = 8.08" for 100-Year event

Inflow = 7.27 cfs @ 12.08 hrs, Volume= 0.550 af

Outflow = 7.27 cfs @ 12.08 hrs, Volume= 0.550 af, Atten= 0%, Lag= 0.0 min

Primary = 7.27 cfs @ 12.08 hrs, Volume= 0.550 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 303.22' @ 12.09 hrs

Flood Elev= 303.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	300.70'	18.0" Round Culvert
	-		L= 35.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 300.70' / 300.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=7.26 cfs @ 12.08 hrs HW=303.21' TW=302.04' (Dynamic Tailwater) 1=Culvert (Inlet Controls 7.26 cfs @ 4.11 fps)

Type III 24-hr WQv Parking Rainfall=2.53"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 8

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1A: Watershed 1A - Parking Runoff Area=35,602 sf 76.59% Impervious Runoff Depth=1.56" Flow Length=190' Tc=5.6 min CN=90 Runoff=1.51 cfs 0.106 af

Pond 3P: Organic Filter (Type F-4) (100% Peak Elev=301.52' Storage=2,520 cf Inflow=1.51 cfs 0.106 af Outflow=0.07 cfs 0.106 af

Pond 6P: Hydrodynamic Separator WQv=1.51cfs, Peak Elev=301.52' Inflow=1.51 cfs 0.106 af 18.0" Round Culvert n=0.013 L=35.0' S=0.0200 '/' Outflow=1.51 cfs 0.106 af

Total Runoff Area = 0.817 ac Runoff Volume = 0.106 af Average Runoff Depth = 1.56" 23.41% Pervious = 0.191 ac 76.59% Impervious = 0.626 ac

Printed 9/26/2018

Page 9

Summary for Subcatchment 1A: Watershed 1A - Parking Lot

Runoff = 1.51 cfs @ 12.08 hrs, Volume= 0.106 af, Depth= 1.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Parking Rainfall=2.53"

	Α	rea (sf)	CN [Description						
		6,211	61 >	61 >75% Grass cover, Good, HSG B						
		2,124	80 >	75% Gras	s cover, Go	ood, HSG D				
*		27,267	98 F	Parking Lot	, Walkways	s, Buildings etc.				
		35,602	90 V	Veighted A	verage					
		8,335		•	rvious Area					
		27,267	7	6.59% Imp	pervious Ar	ea				
		•								
	Тс	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·				
	3.8	12	0.0075	0.05		Sheet Flow, A->B				
						Grass: Dense n= 0.240 P2= 3.41"				
	0.1	5	0.1140	1.56		Sheet Flow, B->C				
						Smooth surfaces n= 0.011 P2= 3.41"				
	1.1	82	0.0151	1.22		Sheet Flow, C->D				
						Smooth surfaces n= 0.011 P2= 3.41"				
	0.6	91	0.0176	2.69		Shallow Concentrated Flow, D->E				
						Paved Kv= 20.3 fps				
	5.6	190	Total							

Summary for Pond 3P: Organic Filter (Type F-4) (100% WQv)

Inflow Area = 0.817 ac, 76.59% Impervious, Inflow Depth = 1.56" for WQv Parking event
Inflow = 1.51 cfs @ 12.08 hrs, Volume= 0.106 af
Outflow = 0.07 cfs @ 15.16 hrs, Volume= 0.106 af, Atten= 96%, Lag= 184.5 min
Outflow = 0.07 cfs @ 15.16 hrs, Volume= 0.106 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 301.52' @ 15.16 hrs Surf.Area= 2,086 sf Storage= 2,520 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 411.7 min (1,227.6 - 815.9)

Volume	Invert Av	ail.Storage	Storage	Description		
#1	300.00'	6,298 cf	Sedime	ntation Basin (Co	nic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft		c.Store c-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
300.00	1,262)	0	0	1,262	
301.00	1,794		1,520	1,520	1,812	
302.00	2,38		2,081	3,601	2,420	
303.00	3,026	;	2,697	6,298	3,091	

Type III 24-hr WQv Parking Rainfall=2.53"

Proposed (2018-03-27)

Prepared by Hudson Engineering & Consulting, P.C.

Printed 9/26/2018

HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Page 10

Device	Routing	Invert	Outlet Devices
#1	Primary	296.98'	12.0" Round 12" HDPE (OUT)
	•		L= 17.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 296.98' / 296.25' S= 0.0429 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	301.60'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	300.00'	1.375 in/hr Exfiltration over Surface area

Primary OutFlow Max=0.07 cfs @ 15.16 hrs HW=301.52' TW=296.41' (Dynamic Tailwater)

-1=12" HDPE (OUT) (Passes 0.07 cfs of 6.00 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

—3=Exfiltration (Exfiltration Controls 0.07 cfs)

Summary for Pond 6P: Hydrodynamic Separator WQv=1.51cfs, 100-year=7.27cfs

0.817 ac, 76.59% Impervious, Inflow Depth = 1.56" for WQv Parking event Inflow Area =

1.51 cfs @ 12.08 hrs, Volume= Inflow 0.106 af

Outflow 1.51 cfs @ 12.08 hrs, Volume= 0.106 af, Atten= 0%, Lag= 0.0 min

1.51 cfs @ 12.08 hrs, Volume= 0.106 af Primary

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 301.52' @ 15.15 hrs

Flood Elev= 303.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	300.70'	18.0" Round Culvert
	-		L= 35.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 300.70' / 300.00' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.51 cfs @ 12.08 hrs HW=301.33' TW=300.72' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.51 cfs @ 2.14 fps)



Hydrodynamic Separator WQv=0.75cfs, 100-year=5.21cfs Watershed 1B - Roof Area









Printed 9/26/2018 Page 2

Area Listing (selected nodes)

Are	a CN	Description
(acres	(a)	(subcatchment-numbers)
0.47	9 98	Proposed Building (1B)
0.47	9 98	TOTAL AREA

Printed 9/26/2018 Page 3

Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.479	Other	1B
0.479		TOTAL AREA

Printed 9/26/2018 Page 4

Ground Covers (selected nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	0.000	0.479	()	Proposed Building	
0.000	0.000	0.000	0.000	0.479	0.479	TOTAL AREA	

Type III 24-hr 100-Year Rainfall=9.29"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 5

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1B: Watershed 1B - Roof Runoff Area=20,885 sf 100.00% Impervious Runoff Depth=9.05" Tc=1.0 min CN=98 Runoff=5.21 cfs 0.362 af

Pond 5P: Hydrodynamic Separator WQv=0.75cfs, Peak Elev=307.02' Inflow=5.21 cfs 0.362 af 12.0" Round Culvert n=0.013 L=123.0' S=0.0587 '/' Outflow=5.21 cfs 0.362 af

Total Runoff Area = 0.479 ac Runoff Volume = 0.362 af Average Runoff Depth = 9.05" 0.00% Pervious = 0.000 ac 100.00% Impervious = 0.479 ac

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 6

Summary for Subcatchment 1B: Watershed 1B - Roof Area

Runoff = 5.21 cfs @ 12.01 hrs, Volume= 0.362 af, Depth= 9.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.29"

	Α	rea (sf)	CN [Description		
*		20,885	98 F	Proposed B	Building	
	20,885 100.00% Impervious Area				Area	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	1.0	•	•			Direct Entry,

Summary for Pond 5P: Hydrodynamic Separator WQv=0.75cfs, 100-year=5.21cfs

Inflow Area = 0.479 ac,100.00% Impervious, Inflow Depth = 9.05" for 100-Year event

Inflow = 5.21 cfs @ 12.01 hrs, Volume= 0.362 af

Outflow = 5.21 cfs @ 12.01 hrs, Volume= 0.362 af, Atten= 0%, Lag= 0.0 min

Primary = 5.21 cfs @ 12.01 hrs, Volume= 0.362 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 307.02' @ 12.01 hrs

Flood Elev= 307.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	303.47'	12.0" Round 12" HDPE (OUT)
	·		L= 123.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 303.47' / 296.25' S= 0.0587 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=5.18 cfs @ 12.01 hrs HW=306.98' TW=298.97' (Dynamic Tailwater) -1=12" HDPE (OUT) (Inlet Controls 5.18 cfs @ 6.59 fps)

Type III 24-hr WQv Roof Rainfall=1.41"

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 7

Time span=0.00-60.00 hrs, dt=0.01 hrs, 6001 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1B: Watershed 1B - Roof Runoff Area=20,885 sf 100.00% Impervious Runoff Depth=1.19" Tc=1.0 min CN=98 Runoff=0.75 cfs 0.048 af

Pond 5P: Hydrodynamic Separator WQv=0.75cfs, Peak Elev=303.97' Inflow=0.75 cfs 0.048 af 12.0" Round Culvert n=0.013 L=123.0' S=0.0587 '/' Outflow=0.75 cfs 0.048 af

Total Runoff Area = 0.479 ac Runoff Volume = 0.048 af Average Runoff Depth = 1.19" 0.00% Pervious = 0.000 ac 100.00% Impervious = 0.479 ac

Prepared by Hudson Engineering & Consulting, P.C. HydroCAD® 10.00-22 s/n 02549 © 2018 HydroCAD Software Solutions LLC

Printed 9/26/2018

Page 8

Summary for Subcatchment 1B: Watershed 1B - Roof Area

Runoff = 0.75 cfs @ 12.01 hrs, Volume= 0.048 af, Depth= 1.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Roof Rainfall=1.41"

	Α	rea (sf)	CN [Description		
*		20,885	98 F	Proposed B	Building	
	20,885 100.00% Impervious Area				Area	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	1.0	•	•			Direct Entry,

Summary for Pond 5P: Hydrodynamic Separator WQv=0.75cfs, 100-year=5.21cfs

Inflow Area = 0.479 ac,100.00% Impervious, Inflow Depth = 1.19" for WQv Roof event

Inflow = 0.75 cfs @ 12.01 hrs, Volume= 0.048 af

Outflow = 0.75 cfs @ 12.01 hrs, Volume= 0.048 af, Atten= 0%, Lag= 0.0 min

Primary = 0.75 cfs @ 12.01 hrs, Volume= 0.048 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-60.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 303.97' @ 12.01 hrs

Flood Elev= 307.98'

Device	Routing	Invert	Outlet Devices
#1	Primary	303.47'	12.0" Round 12" HDPE (OUT)
	,		L= 123.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 303.47' / 296.25' S= 0.0587 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.74 cfs @ 12.01 hrs HW=303.97' TW=296.69' (Dynamic Tailwater) 1=12" HDPE (OUT) (Inlet Controls 0.74 cfs @ 1.90 fps)

10.) State of Maryland Department of the Environment (MDE) List of Approved Stormwater Practices (August 2017)



Larry Hogan Governor

Boyd Rutherford Lieutenant Governor

Ben Grumbles Secretary

Alternative/Innovative Technology List of Approved Stormwater Practices (August 2017)

Practice Name	Manufacturer	Practice Type	Approval Type	BMP Category	BMP Code	Approval Date
StormCap™	Flex Membrane International /Stormwater Capture Co.	Green Roof	Alternative Surface	Α	AGRE	7 /21/2017
Silva Cell Pavement System	DeepRoot Green Infrastructure LLC	Bioretention/Pavement System	ESD-All, Structural WQv, Structural Component	E, S	MMBR, FBIO	6 /16/2017
KBI FlexiPave	K.B. Industries	Permeable Pavement	Alternative Surface	Α	APRP	5 /17/2017
StormTreat System	StormTreat Systems, Inc.	Submerged Gravel Wetland & Bioretention/Filter	ESD-All, Structural WQv	E, S	MSGW, MMBR, FBIO	5 /15/2017
LiveRoof Hybrid Green Roof System	LiveRoof Global, LLC	Green Roof	Alternative Surface	Α	AGRE	2 /28/2017
StormPro	Environment 21, LLC	Hydrodynamic Separator	Pretreatment	X	XOGS	2 /7 /2017
VR Max Vegetated Roof System	Tremco Incorporated	Green Roof	Alternative Surface	А	AGRE	11/4 /2016
FocalPoint Bioretention Systems	ACF-Convergent Alliance	Bioretention	MS4 Retrofit, ESD WQv Only	E, S	MMBR, FBIO	9 /8 /2016
Suntree Nutrient Separating Baffle Box	Suntree Technologies	Hydrodynamic Separator	Pretreatment	S	XOGS	9 /8 /2016

Practice Name	Manufacturer	Practice Type	Approval Type	BMP Category	BMP Code	Approval Date
Columbia Green Technologies Green Roof Systems	Columbia Green Roof Technologies	Green Roof System	Alternative Surface	Α	AGRE	9 /2 /2016
PaverGuide	PaverGuide, Inc.	Base/Storage Reservoir for Permeable Pavers	Alternative Surface	Α	APRP	8 /29/2016
HydroBlox	HydroBlox Technologies, Inc.	Drainage/Conveyance Alternative	Structural Component	Х	хотн	5 /31/2016
Henry Green Roof Products	Henry Company	Green Roof System	Alternative Surface	Е	AGRE	2 /5 /2016
Opti RTC Continuous Monitoring and Adaptive Control (CMAC)	OptiRTC, Inc.	Structural control component for wet ponds	Structural Component	X	ХОТН	1 /27/2016
PerkFilter	Oldcastle Precast	Cartridge (Sand) Filter	Structural WQv	S	FSND	9 /16/2015
Hydropack Green Roof System	Vegetal i.D. Inc.	Green Roof	Alternative Surface	Α	AGRE	9 /10/2015
Modular Wetland System - Linear	Modular Wetland Systems, Inc.	Bioretention/Micro- Bioretention/Submerged Gravel Wetland	MS4 Retrofit, ESD WQv Only, Structural WQv	E, S	MMBR, MSWG, FBIO	9 /8 /2015
AWD SITEDRAIN Strip 9624	American Wick Drain	Underdrain Alternative	Structural Component	Х	XOTH	4 /6 /2015
MP Eco-Grid	USA EcoSystems	Reinforced Turf System	Alternative Surface	Е	ARTF	1 /22/2015
Rotondo Bio-Filter	Rotondo Env. Solutions, LLC	Bioretention System	MS4 Retrofit	E, S	MMBR, FBIO	1 /9 /2015
Hydrotech Green Roofing System	American Hydrotech, LLC	Green Roof System	Alternative Surface	Е	AGRE	1 /9 /2015
Stormcrete	Porous Technologies, LLC	Permeable Pavement	Alternative Surface	Е	AGRE	12/9 /2014
Green Roof Outfitters Modular Roof System	Green Roof Outfitters, LLC	Modular Green Roof	Alternative Surface	Е	AGRE	11/20/2014

Practice Name	Manufacturer	Practice Type	Approval Type	BMP Category	BMP Code	Approval Date
Eco-Roof	Eco-Roofs, LLC	Green Roof System	Alternative Surface	Е	AGRE	4 /18/2014
StormTank StormShield	Brentwood Industries, Inc.	Vault/Filter System	Pretreatment	S	XOGS	3 /5 /2014
Rotondo Bio-Pod	Rotondo Env. Solutions, LLC	Permeable Pavement/Vault System	Pretreatment	S	хотн	1 /7 /2014
AquaLok GLU	FGP Enterprises, LLC	Rainwater Harvesting	ESD-All	E	MRWH	1 /7 /2014
Clay Brick Pavers	The Brick Industry	Permeable Pavement	Alternative Surface	E	APRP	8 /12/2013
CrystalClean Separator	CrystalStream Technologies	Hydrodynamic Device	Pretreatment	S	XOGS	5 /30/2013
Aqua Bric/Bio-Pave	Filterra Bioretention Systems	Interlocking Paving System	Alternative Surface, ESD-All, Structural WQv	E	APRP	3 /19/2013
SAFL Baffle	Upstream Technologies	OGS/Filter System	Pretreatment	S	XOGS	3 /12/2013
COREgravel	Core Systems	Reinforced Turf	Alternative Surface	Е	ARTF	3 /12/2013
EZ Roll Grass and Gravel Pavers	NDS, Inc.	Reinforced Turf	Alternative Surface	Е	ARTF	3 /12/2013
EcoCline Living Roof System	Furbish Company	Green Roof	Alternative Surface	Е	AGRE, AGRI	2 /25/2013
Filterra Bioretention System	Filterra Bioretention Systems	Bioretention	ESD WQv Only, Structural WQv	E, S	MMBR, FBIO	2 /22/2013
Grasscrete	Storm-Services, LLC	Reinforced Turf	Alternative Surface	Е	ARTF	12/3 /2012
Nicolock Pavers	Nicolock Paving Stones	Permeable Paver	Alternative Surface	Е	APRP	8 /3 /2012
AquaLok Panels	FGP Enterprises, LLC	Green Roof/ Rainwater Harvesting	Alternative Surface, ESD-All	Е	AGRE, MRWH	6 /20/2012

Practice Name	Manufacturer	Practice Type	Approval Type	BMP Category	BMP Code	Approval Date
PaveDrain	Ernest Maier, Inc.	Permeable Pavement	Alternative Surface	Е	APRP	3 /29/2012
Jellyfish Filter	Imbrium Systems Corporation	Cartridge/Membrane Filter	Structural WQv	S	FUND	3 /12/2012
Floating Treatment Wetlands	BlueWing Env. Solutions	Modular Wetland	Pretreatment	S	XOTH	3 /8 /2012
StormBasin	Fabco Industries, Inc.	OGS/Filter	Pretreatment	S	XOGS	2 /13/2012
StormSafe	Fabco Industries, Inc.	Vault/Filter System	Pretreatment	S	XOGS	2 /13/2012
StormSack	Fabco Industries, Inc.	Catch Basin Insert	Pretreatment	S, A	XOGS, CBC	2 /13/2012
PhosphoSorb Media	ConTech Construction	Filter Media	Structural WQv	S	FUND	11/18/2011
BaySeparator	BaySaver Technologies, Inc.	Hydrodynamic Device	Pretreatment	S	XOGS	8 /10/2011
FlexStorm	Nyloplast	Catch Basin Insert	Pretreatment	S, A	XOGS, CBC	5 /17/2011
V2B1 Hydrodynamic Separator	Environment 21	Hydrodynamic Device	Pretreatment	S	XOGS	10/6 /2010
Flo-Gard	Oldcastle Precast	Inlet Filter	Pretreatment	S	XOTH	8 /19/2010
Sorbtive Media	Imbrium Systems Corporation	Filtering Media	Structural WQv	S	ND, FPER, FO	10/21/2009
Sorbtive Filter	Imbrium Systems Corporation	Filter	Structural WQv	S	ND, FUND, FP	9 /11/2009
UrbanGreen	Contech Construction Product	Filter	Structural WQv	S	FBIO	6 /3 /2009
StormTank	Brentwood Industries	Storage Tank	Pretreatment	S	XFLD	11/6 /2008
FloGard Dual Vortex Separator (DVS)	Oldcastle Precast	Hydrodynamic Device	Pretreatment	Х	XOGS	3 /25/2008
ADS/Hancor WQU	ADS Hancor	Hydrodynamic Device	Pretreatment	S	XOGS	3 /25/2008
StormTech Isolator	StormTech, LLC	Storage Tank	Structural Component	S	XFLD	11/7 /2007
No Fault/Smarte Surface	Human & Rohde	Permeable Surfaces	Alternative Surface	Е	APRP, ARTF	6 /1 /2007
Flo-Guard Plus	Oldcastle	Catch Basin Insert	Pretreatment	S, A	XOGS, CBC	3 /27/2007

Practice Name	Manufacturer	Practice Type	Approval Type	BMP Category	BMP Code	Approval Date
Up-Flo Filter	Hydro International	Catch Basin Insert	Pretreatment	S, A	XOGS, CBC	2 /6 /2007
Storm-Pure	Nyloplast	Catch Basin Insert	Pretreatment	S, A	XOGS, CBC	11/20/2006
BayFilter	BaySaver Technologies, Inc.	Cartridge Filter	Structural WQv	S	FUND	10/12/2006
Aqua Swirl	AquaShield, Inc.	Hydrodynamic Device	Pretreatment	S	XOGS	5 /5 /2006
Stormfilter	Stormwater Management, Inc.	Cartridge Filter	Structural WQv	S	FUND	4 /11/2005
Terre Kleen	Terre Hill Concrete Products	Hydrodynamic Device	Pretreatment	S	XOGS	3 /28/2005
Ultra-Urban Filter	Abtech Industries	Catch Basin Insert	Pretreatment	S, A	XOGS, CBC	2 /15/2005
Vortfilter	Vortechnics, Inc.	Cartridge Filter	Pretreatment	S	FUND	1 /6 /2005
CDS Media Filtration System	CDS Technologies, Inc	Cartridge Filter	Structural WQv	S	FUND	12/30/2004
FirstDefense	Hydro International	Hydrodynamic Device	Pretreatment	S	XOGS	11/30/2004
Vortechs & Vort Sentry	Vortechnics, Inc.	Hydrodynamic Device	Pretreatment	S	XOGS	6 /1 /2004
Downstream Defender	Hydro International	Hydrodynamic Device	Pretreatment	S	XOGS	5 /4 /2004
CDS Oil / Grit Separator	CDS Technologies, Inc	Hydrodynamic Device	Pretreatment	S	XOGS	8 /15/2003
Aqua Filter	AquaShield, Inc.	Cartridge Filter	Structural WQv	S	FUND	6 /23/2003
BaySaver	BaySaver, Inc.	Hydrodynamic Device	Pretreatment	S	XOGS	6 /11/2002
Stormceptor	Imbrium Systems Corporation	Hydrodynamic Device	Pretreatment	S	XOGS	4 /16/2001

Please contact each vendor/manufacturer for approval letters and more specific product information for each of the above-listed practices. Any formal request to MDE concerning an alternative/innovative technology should be submitted to MDE's Sediment, Stormwater, and Dam Safety Program, 1800 Washington Boulevard, Baltimore, MD 21230. If there are any questions concerning these practices, please contact the Maryland Department of the Environment, Water and Science Administration at 410-537-3543 or at www.mde.maryland.gov.

11.) Stormwater Management Construction Checklists

APPENDIX H

STATE POLLUTANT DISCHARGE ELIMINATION SYSTEM FOR CONSTRUCTION ACTIVITIES CONSTRUCTION SITE LOG BOOK

Table of Contents

- I. Pre-Construction Meeting Documents
 - a. Preamble to Site Assessment and Inspections
 - b. Operator's Certification
 - c. Qualified Professional's Credentials & Certification
 - d. Pre-Construction Site Assessment Checklist
- II. Construction Duration Inspections
 - a. Directions
 - b. Modification to the SWPPP
- III. Monthly Summary Reports
- IV. Monitoring, Reporting, and Three-Month Status Reports
 - a. Operator's Compliance Response Form

Properly completing forms such as those contained in Appendix H meet the inspection requirement of NYS-DEC SPDES GP for Construction Activities. Completed forms shall be kept on site at all times and made available to authorities upon request.

I. PRE-CONSTRUCTION MEETING DOCUMENTS Project Name Permit No. _____ Date of Authorization ______ Name of Operator _____ Prime Contractor

a. Preamble to Site Assessment and Inspections

The Following Information To Be Read By All Person's Involved in The Construction of Stormwater Related Activities:

The Operator agrees to have a qualified professional¹ conduct an assessment of the site prior to the commencement of construction² and certify in this inspection report that the appropriate erosion and sediment controls described in the SWPPP have been adequately installed or implemented to ensure overall preparedness of the site for the commencement of construction.

Prior to the commencement of construction, the Operator shall certify in this site logbook that the SWPPP has been prepared in accordance with the State's standards and meets all Federal, State and local erosion and sediment control requirements.

When construction starts, site inspections shall be conducted by the qualified professional at least every 7 calendar days and within 24 hours of the end of a storm event of 0.5 inches or greater (Construction Duration Inspections). The Operator shall maintain a record of all inspection reports in this site logbook. The site logbook shall be maintained on site and be made available to the permitting authorities upon request. The Operator shall post at the site, in a publicly accessible location, a summary of the site inspection activities on a monthly basis (Monthly Summary Report).

The operator shall also prepare a written summary of compliance with this general permit at a minimum frequency of every three months (Operator's Compliance Response Form), while coverage exists. The summary should address the status of achieving each component of the SWPPP.

Prior to filing the Notice of Termination or the end of permit term, the Operator shall have a qualified professional perform a final site inspection. The qualified professional shall certify that the site has undergone final stabilization³ using either vegetative or structural stabilization methods and that all temporary erosion and sediment controls (such as silt fencing) not needed for long-term erosion control have been removed. In addition, the Operator must identify and certify that all permanent structures described in the SWPPP have been constructed and provide the owner(s) with an operation and maintenance plan that ensures the structure(s) continuously functions as designed.

^{1 &}quot;Qualified Professional means a person knowledgeable in the principles and practice of erosion and sediment controls, such as a Certified Professional in Erosion and Sediment Control (CPESC), soil scientist, licensed engineer or someone working under the direction and supervision of a licensed engineer (person must have experience in the principles and practices of erosion and sediment control).

^{2 &}quot;Commencement of construction" means the initial removal of vegetation and disturbance of soils associated with clearing, grading or excavating activities or other construction activities.

^{3 &}quot;Final stabilization" means that all soil-disturbing activities at the site have been completed and a uniform, perennial vegetative cover with a density of eighty (80) percent has been established or equivalent stabilization measures (such as the use of mulches or geotextiles) have been employed on all unpaved areas and areas not covered by permanent structures.

b. Operators Certification

Name (please print):

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. Further, I hereby certify that the SWPPP meets all Federal, State, and local erosion and sediment control requirements. I am aware that false statements made herein are punishable as a class A misdemeanor pursuant to Section 210.45 of the Penal Law.

Title		Date:	
Signature:		** ***********************************	
c. Qualified Profess	ional's Credentials & Cert	ification	
project and that the ap	propriate erosion and sediment struction Site Assessment Che	the General Permit to conduct site ins nt controls described in the SWPPP ar ecklist have been adequately installed e commencement of construction."	nd as described in
Name (please print):			
		Date:	
Address:			
Signature:			

d. Pre-construction Site Assessment Checklist (NOTE: Provide comments below as necessary)

1. Notice of Intent, SWPPP, and Contractors Certification:
Yes No NA
[] [] Has a Notice of Intent been filed with the NYS Department of Conservation? [] [] Is the SWPPP on-site? Where?
[] [] Is the Plan current? What is the latest revision date?
[] [] Is a copy of the NOI (with brief description) onsite? Where?
[] [] Have all contractors involved with stormwater related activities signed a contractor's certification?
2. Resource Protection
Yes No NA
 [] [] Are construction limits clearly flagged or fenced? [] [] Important trees and associated rooting zones, on-site septic system absorption fields, existing vegetated areas suitable for filter strips, especially in perimeter areas, have been flagged for protection.
[] [] Creek crossings installed prior to land-disturbing activity, including clearing and blasting.
3. Surface Water Protection
Yes No NA
[] [] Clean stormwater runoff has been diverted from areas to be disturbed.
[] [] Bodies of water located either on site or in the vicinity of the site have been identified and protected.
[] [] Appropriate practices to protect on-site or downstream surface water are installed. [] [] Are clearing and grading operations divided into areas <5 acres?
4. Stabilized Construction Entrance
Yes No NA
[] [] A temporary construction entrance to capture mud and debris from construction vehicles before they enter the public highway has been installed.
[] [] Other access areas (entrances, construction routes, equipment parking areas) are stabilized immediately as work takes place with gravel or other cover.
[] [] Sediment tracked onto public streets is removed or cleaned on a regular basis.
5. Perimeter Sediment Controls
Yes No NA
[] [] Silt fence material and installation comply with the standard drawing and specifications.
[] [] Silt fences are installed at appropriate spacing intervals
[] [] Sediment/detention basin was installed as first land disturbing activity.
[] [] Sediment traps and barriers are installed.
6. Pollution Prevention for Waste and Hazardous Materials Yes No NA
[] [] The Operator or designated representative has been assigned to implement the spill prevention
avoidance and response plan.
[] [] The plan is contained in the SWPPP on page
[] [] Appropriate materials to control spills are onsite. Where?
•

II. CONSTRUCTION DURATION INSPECTIONS

a. Directions:

Inspection Forms will be filled out during the entire construction phase of the project. Required Elements:

- (1) On a site map, indicate the extent of all disturbed site areas and drainage pathways. Indicate site areas that are expected to undergo initial disturbance or significant site work within the next 14-day period;
- (2) Indicate on a site map all areas of the site that have undergone temporary or permanent stabilization;
- (3) Indicate all disturbed site areas that have not undergone active site work during the previous 14-day period;
- (4) Inspect all sediment control practices and record the approximate degree of sediment accumulation as a percentage of sediment storage volume (for example, 10 percent, 20 percent, 50 percent);
- (5) Inspect all erosion and sediment control practices and record all maintenance requirements such as verifying the integrity of barrier or diversion systems (earthen berms or silt fencing) and containment systems (sediment basins and sediment traps). Identify any evidence of rill or gully erosion occurring on slopes and any loss of stabilizing vegetation or seeding/mulching. Document any excessive deposition of sediment or ponding water along barrier or diversion systems. Record the depth of sediment within containment structures, any erosion near outlet and overflow structures, and verify the ability of rock filters around perforated riser pipes to pass water; and
- (6) Immediately report to the Operator any deficiencies that are identified with the implementation of the SWPPP.

CONSTRUCTION DURATION INSPECTIONS Page 1 of _____ SITE PLAN/SKETCH **Date of Inspection** Inspector (print name) **Qualified Professional Signature Qualified Professional (print name)** The above signed acknowledges that, to the best of his/her knowledge, all information provided on the

forms is accurate and complete.

Maintaining Water Quality

Yes No NA	
[] [] [] Is there an increase in turbidity causing a substantial visible contra [] [] [] Is there residue from oil and floating substances, visible oil film, o [] [] All disturbance is within the limits of the approved plans. [] [] [] Have receiving lake/bay, stream, and/or wetland been impacted by	r globules or grease?
Housekeeping	
1. General Site Conditions	
Yes No NA	
 [] [] Is construction site litter and debris appropriately managed? [] [] Are facilities and equipment necessary for implementation of erosi working order and/or properly maintained? 	on and sediment control in
[] [] Is construction impacting the adjacent property? [] [] Is dust adequately controlled?	
2. Temporary Stream Crossing	
Yes No NA	
[] [] Maximum diameter pipes necessary to span creek without dredgin [] [] [] Installed non-woven geotextile fabric beneath approaches. [] [] [] Is fill composed of aggregate (no earth or soil)?	
[] [] Rock on approaches is clean enough to remove mud from vehicles entering stream during high flow.	& prevent sediment from
Runoff Control Practices	
1. Excavation Dewatering	
Yes No NA	
[] [] Upstream and downstream berms (sandbags, inflatable dams, etc.) [] [] [] Clean water from upstream pool is being pumped to the downstrea [] [] [] Sediment laden water from work area is being discharged to a silt- [] [] [] Constructed upstream berm with one-foot minimum freeboard.	m pool.
2. Level Spreader	
Yes No NA	
[] [] [] Installed per plan.	4: aut 1a dau flass.
[] [] Constructed on undisturbed soil, not on fill, receiving only clear, n [] [] Flow sheets out of level spreader without erosion on downstream of	
3. Interceptor Dikes and Swales	
Yes No NA	
[] [] Installed per plan with minimum side slopes 2H:1V or flatter.	
[] [] Stabilized by geotextile fabric, seed, or mulch with no erosion occ [] [] [] Sediment-laden runoff directed to sediment trapping structure	urring.

Page 3 of _____ CONSTRUCTION DURATION INSPECTIONS **Runoff Control Practices (continued)** 4. Stone Check Dam. Yes No NA [] [] Is channel stable? (flow is not eroding soil underneath or around the structure). [] [] Check is in good condition (rocks in place and no permanent pools behind the structure). [] [] [] Has accumulated sediment been removed?. 5. Rock Outlet Protection Yes No NA [] [] Installed per plan. [] [] Installed concurrently with pipe installation. Soil Stabilization 1. Topsoil and Spoil Stockpiles Yes No NA [] [] Stockpiles are stabilized with vegetation and/or mulch. [] [] Sediment control is installed at the toe of the slope. 2. Revegetation Yes No NA [] [] Temporary seedings and mulch have been applied to idle areas. [] [] 4 inches minimum of topsoil has been applied under permanent seedings **Sediment Control Practices** 1. Stabilized Construction Entrance Yes No NA [] [] Stone is clean enough to effectively remove mud from vehicles. [] [] Installed per standards and specifications? [] [] Does all traffic use the stabilized entrance to enter and leave site? [] [] Is adequate drainage provided to prevent ponding at entrance? 2. Silt Fence

[] [] Installed on Contour, 10 feet from toe of slope (not across conveyance channels).
[] [] Joints constructed by wrapping the two ends together for continuous support.

[] [] Posts are stable, fabric is tight and without rips or frayed areas.

[] [] Fabric buried 6 inches minimum.

Sediment accumulation is ____% of design capacity.

Yes No NA

CONSTRUCTION DURATION INSPECTIONS

Page 4 of _____

Sediment Control Practices (continued)

3. Storm Drain Inlet Protection (Use for Stone & Block; Filter Fabric; Curb; or, Excavated practices) Yes No NA
 [] [] Installed concrete blocks lengthwise so open ends face outward, not upward. [] [] Placed wire screen between No. 3 crushed stone and concrete blocks. [] [] Drainage area is 1 acre or less. [] [] Excavated area is 900 cubic feet. [] [] Excavated side slopes should be 2:1. [] [] [] 2" x 4" frame is constructed and structurally sound. [] [] Posts 3-foot maximum spacing between posts. [] [] Fabric is embedded 1 to 1.5 feet below ground and secured to frame/posts with staples at max 8-inch spacing. [] [] Posts are stable, fabric is tight and without rips or frayed areas.
Sediment accumulation% of design capacity.
4. Temporary Sediment Trap
Yes No NA [] [] Outlet structure is constructed per the approved plan or drawing. [] [] Geotextile fabric has been placed beneath rock fill. Sediment accumulation is% of design capacity.
5. Temporary Sediment Basin
Yes No NA
[] [] Basin and outlet structure constructed per the approved plan. [] [] Basin side slopes are stabilized with seed/mulch. [] [] Drainage structure flushed and basin surface restored upon removal of sediment basin facility. Sediment accumulation is% of design capacity.
Note: Not all erosion and sediment control practices are included in this listing. Add additional pages to this list as required by site specific design. Construction inspection checklists for post-development stormwater management practices can be found in Appendix F of the New York Stormwater Management Design Manual.

CONSTRUCTION DURATION INSPECTIONS

b. Modifications to the SWPPP (To be completed as described below)

The Operator shall amend the SWPPP whenever:

- 1. There is a significant change in design, construction, operation, or maintenance which may have a significant effect on the potential for the discharge of pollutants to the waters of the United States and which has not otherwise been addressed in the SWPPP; or
- 2. The SWPPP proves to be ineffective in:
 - a. Eliminating or significantly minimizing pollutants from sources identified in the SWPPP and as required by this permit; or
 - b. Achieving the general objectives of controlling pollutants in stormwater discharges from permitted construction activity; and
- 3. Additionally, the SWPPP shall be amended to identify any new contractor or subcontractor that will implement any measure of the SWPPP. Modification & Reason:

III. Monthly Summary of Site Inspection Activities

Name of Permit	ted Facility:	Today's Date:	Reporting Month:	
Location:			Permit Identificati	on #:
Name and Telep	hone Number of Site Inspec	ctor:		
Date of Inspection	Regular / Rainfall based Inspection	Name of Inspecto	or Ite	ems of Concern
Owner/Opera	tor Certification:			
accordance with submitted. Based gathering the info	enalty of law that this docum a system designed to assure the on my inquiry of the person formation, the information sub ware that false statements made."	hat qualified personnel pro or persons who manage the mitted is, to the best of my	perly gathered and eves system, or those per knowledge and belie	aluated the information sons directly responsible for f, true, accurate, and
Signature of Permi	ttee or Duly Authorized Represe	ntative Name of Po	ermittee or Duly Author	ized Representative Date
Duly authorized documents.	representatives must have	e written authorization, s	ubmitted to DEC, to	sign any permit

NYSDEC SPDES General Permit for Stormwater Discharges from Construction Activity Monthly Summary of Site Inspection Activities Permit Number GP-02-01

Name of Permitted Facility:		Permit Identification #:	
Location:	•	Foday's Date:	Reporting Month:
Name and Telephone Number of Site Inspector:	Name and Telephone Number of Site Inspector:	ımber of Site Inspector:	

Permit Reference; Part III.D.3.b (page 15):

"The onergor shall nost at the site in a publicly-accessible location a summary of the site inspection activities

	_			ı —	1	ľ			_	Γ	
· .	Date	Corrected									
a publicity-accessione tocation, a summary of the site inspection activities on a moninty basis.	Major items of concern related to compliance of the	SWPPP with all conditions of the general permit									
ine sue, in a publiciy-accessible tocam	Name of Qualified Professional	conducting Site Inspections									
the operator shall post at the site, in	Type of Inspection										
7	Date of	Inspection									

Owner/Operator Certification:

persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware "I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those that false statements made herein are punishable as a class A misdemeanor pursuant to Section 210.45 of the Penal Law."

Date	ny permit documents.
Duly Authorized Representative	zation, submitted to DEC, to sign a
Name of Permittee or I	tor) must have written authoriz
Signature of Permittee or Duly Authorized Representative	Duly authorized representatives of the Permittee (Owner/Operat

Inspection and Maintenance Checklist Catch Basins, Manholes, and Inlets

Date:									
Type of Inspection:	Storm		Weekly		Monthly		Annual 🗌		
Site:	ite: Inspector(s):								
Description or location of Project:									

	Pro de consuleros de calculativamento de calcu		
	Conditions when Maintenance	Maintenance	
Defect General	is Needed		Comments
Trash and Debris	Trash and debris which are		
Hasif and Debits	located immediately in front of		
	the catch basin opening or is		
	blocking inletting capacity of the		
	basin by more than 10%.		
	Trash or debris (in the basin) that		
	exceeds 60 percent of the sump		
	depth as measured from the		
	bottom of basin to invert of the		
	lowest pipe into or out of the		
	basin, but in no case less than a		
	minimum of six inches clearance		
	from the debris surface to the		
	invert of the lowest pipe.		
	Trash or debris in any inlet or		
	1		
	outlet pipe blocking more then		
	1/3 of its height.		
	Dead animals or vegetation that		
	could generate odors that could		
	cause complaints or dangerous		
S 42 I	gases (e.g., methane).		
Sediment	Sediment (in the basin) that		
	exceeds 60 percent of the sump		
	depth as measured from the		
	bottom of basin to invert of the		
	lowest pipe into or out of the		
	basin, but in no case less than a		
	minimum of 6 inches clearance		
	from the sediment surface to the		
	invert of the lowest pipe.		
Structure Damage to	Top slab has holes larger than 2		
Frame and/or Top Slab	square inches or cracks wider		
	then ¼ inch.		
	Frame not sitting flush on top		
	slab, i.e., separation of more		
	than ¾ inch of the frame from		
	the top slab. Frame not securely		
	attached.		

^{*}Maintenance: Enter 1 if maintenance is needed. Enter 2 if maintenance was preformed same day.

Defect:	Conditions when Maintenance Is Needed	Maintenance:	Comments
Fractures or Cracks in	Maintenance person judges that		
Basin Walls/Bottom	structure is unsound.		
	Grout fillet has separated or		
	cracked wider then ½ inch and		
	longer than 1 foot at the joint of		
	any inlet/outlet pipe or any		
	evidence of soil particles		
	entering catch basin through		
	cracks.		
Settlement/Misalignment	If failure of basin has created a		
, , ,	safety, function, or design		
	problem.		
Vegetation	Vegetation growing across and		
Vegetation	blocking more than 10% of the		
	basin opening.		
	Vegetation growing in		
	inlet/outlet pipe joints that is		
	more than 6 inches tall and less		
	than 6 inches apart.		
Contamination and	*		
Pollution	Any evidence of oil, gasoline, contaminants or other		
Pollution			
	pollutants.		
Catch Basin Cover			
Cover Not in Place	Cover is missing or only partially		
	in place. Any open catch basin		
	requires maintenance.		
Locking Mechanism Not	Mechanism cannot be opened by		
Working	one maintenance person with		
	proper tools. Bolts into frame		
	have less than 1/2 inch of thread.		
Cover Difficult to Remove	One maintenance person cannot		
	remove lid after applying normal		
	lifting pressure.		
	(Intent is keep cover from sealing		
	off access to maintenance).		
Ladder			
Ladder Rungs Unsafe	Ladder is unsafe due to missing		
	rungs, not securely attached to		
	basin wall, misalignment, rust,		
	cracks, or sharp edges.		
Metal Grates (If Applicable			
Grate opening Unsafe	Grate with opening wider than		
, 5	7/8 inch.		
Trash and Debris	Trash and debris that is blocking		
	more than 20% of grate surface		
	inletting capacity.	ĺ	
Damaged or Missing	Grate missing or broken		
Damagea of Missing	member(s) of the grate.		
	member(s) or the grate.		

^{*}Maintenance: Enter 1 if maintenance is needed. Enter 2 if maintenance was preformed same day.

Inspection and Maintenance Checklist Conveyance Systems (Pipes & Ditches)

Date:									
Type of Inspection:	Storm		Weekly		Monthly		Annual		
Site:	lnspector(s):								

		POSINO HAMBITE MADE IN A STATE OF THE STATE	
	Conditions When Maintenance	Måintenance».	The Parties of the confidence
Defect	Is Needed	1 (1 of 2)	Comments &
Pipes			
Sediment &	Accumulated Sediment that		
Debris	exceeds 20% of the diameter of		
	the pipe.		
Vegetation	Vegetation that reduces free		
	movement of water through		
	pipes		
Damaged Pipe	Protective coating is damaged;		
	rust is causing more than 50%		
	deterioration to any part of		
	pipe.		
	Any dent that decreases the		
	cross section area of pipe by more than 20% or puncture that		
	impacts performance.		
Open Ditches	impacts performance.		
Trash and Debris	Trash and debris > 5 cf/1000 sf		
Trasti and Bestis	(one standard size garbage can)		
	(one standard size garbage carry		
	Visual evidence of dumping		
Sediment	Accumulated sediment that		
	exceeds 20% of the design		
	depth.		
Vegetation	Vegetation that reduces free		
	movement of water through		
	ditches.		
Erosion Damage	Eroded damage over 2 inches		
to Slopes and	deep where cause of damage is		
Channel Bottom	still present or where there is		
	potential for continued erosion.		
Rock Lining Out of	Maintenance person can see		
Place or Missing	native soil beneath the rock		
(If Applicable)	lining.		

^{*}Maintenance: Enter 1 if maintenance is needed. Enter 2 if maintenance was preformed same day.